Research Article

Optimal Skip-Stop Schedule under Mixed Traffic Conditions for Minimizing Travel Time of Passengers

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1. Introduction

In the recent years, with the rapid development of public transport, bus stops face an increasing pressure especially during peak hours, their efficiency decreases continuously, and even serious traffic congestion occurs frequently. The fundamental reason for the above problems is that bus demand exceeds the capacity of bus stops, resulting in some buses waiting in the travel lane until the buses occupying the berth entrance. This not only decreases the level of public transit service but also increases the impact between buses and cars at the location of bus stops on the road without exclusive bus lane. With skip-stop bus services that serve only a subset of stops along certain routes, this problem can be alleviated without technological improvement. For bus passengers, skip-stop services mean improved service levels in the form of lower travel time due to fewer stops and higher between-stop speed. When the skip-stop schedule is adopted by buses, it will reduce the impact of stopping buses on the cars at the location of bus stops, which will increase the car users’ travel speed. In actual practice, skip-stop services in systems such as Transmilenio (Bogota, Colombia) and Metro Rapid (Los Angeles, CA, USA) have been proven to be highly effective [1–3]. In China, Shanghai and Shenzhen have also started implementing this operating strategy. However, most cities in China use regular service which is the all-stop operational scheme considering the operational complexity and lack of methodology in modeling an optimal operational scheme.

The literature has a considerable amount of work on the transit operation optimization. Eberlein [4] formulated the stop-skipping problem as an integer nonlinear programming model with both quadratic objective functions and constraints. The real-time scheduling strategy aiming to strike an optimal balance between the benefits of operators and passengers was formulated by Fu et al. [5]. In this study, the problem is again formulated as a nonlinear 0-1 integer programming problem. Leiva et al. [3] developed an optimization method for designing skip-stop services that minimize the social costs of a segregated bus lane assuming known trip demand. An optimization method is proposed for designing skip-stop services which minimize both the travel time for passengers and the operating cost of an urban bus corridor by optimizing the transport services with different arriving frequencies of various types of buses [6]. In addition, Ceder and Wilson [7], Fan and Machemehl [8], and...
Mauttone and Urquhart [9] determined a set of routes and
their respective frequencies by minimizing the sum of users
and transit operator costs in a given network and trip demand
O-D matrix.

In those studies, the optimization methods are developed
inside the transit system, while ignoring the interaction of
private and transit vehicle flows. However, there are some
roads without exclusive bus lanes in the cities of China.
Ignoring this will lead to inaccurate estimates of travel time.
Moreover, most of the previous studies were done only for a
congested bus line, and they lack the analysis of the overall
bus lines that passed by the stops. With these arguments
as motivation, this study proposes an optimization method
for designing skip-stop services that can minimize the total
travel time for passengers under mixed traffic conditions by
analyzing the mutual influence between buses and cars, and
it uses genetic algorithm to find the optimal coordination of
the stop stations of bus lines based on the objective function.

The following notations are used in describing the models
in this paper:

- $C_i$ is the capacity of link; $C_0$ is the basic traffic capacity;
- $f_s$ is the adjustment factor for bus stops; $T_e$ is the impacting time by bus stops, which is deter-
  mined by the number of stopping buses at this stop;
- $p$ is the probability of no bus being serviced; $\tau$ is the average dwell time of buses;
- $\lambda$ is the average bus arrival rate; $\lambda_1$ is the average arrival rate of buses that stopped at this
  stop.
- $S$ is the number of loading areas; $N$ is the number of lanes in one direction, $N \geq 2$;
- $T(q)$ is the average travel time of vehicle on link $a$; $T_0^i$ is the free-flow travel time on link $a$;
- $q$ is the volume of traffic on link $i$; $\alpha, \beta$ are parameters;
  - $i$ is the index of bus line, $i = 0, 1, \ldots, m$;
  - $I$ is the set of all lines in the network;
  - $A$ is the set of all links in the network;
  - $J$ is the set of all stations stopped and skipped by bus
    line $i$;
- $q_a^{bc}$ is the bus volume or car volume on link $a$;
- $t^c_a$ is the in-vehicle travel time on link $a$ by mode car;
- $t^b_{jk}$ is the in-vehicle travel time on link $k$ by mode bus;
- $n^b_{ik}$ is the average number of passengers on bus line $i$ on
  link $k$;
- $n^c_s$ is the average number of passengers on car;
- $W_j$ is the total waiting time for passengers at stop $j$, which
  includes alighting and transferring at stop $j$;
- $K_i$ is the set of all links where bus line $i$ runs;
- $Q_{ij}$ is the number of passengers on buses of line $i$ at stop $j$;
- $\tau_{ij}$ is the dwell time of buses of line $i$ at stop $j$;
- $U_{ij}$ is the number of passengers boarding buses of line $i$ at stop $j$;
- $V_{ij}$ is the number of passengers alighting buses of line $i$ at stop $j$;
- $w_{ij}$ is the waiting time of passengers boarding buses of
  line $i$ at stop $j$;
- $y_{ij}$ is the decision variables to indicate stop status of bus
  line $i$ at stop $j$, which is equal to 1 if bus line $i$ stops at
  station $j$ and 0 otherwise;
- $L_j$ is the distance between station $j$ and $j + 1$;
- $f_i$ is the schedule frequency of bus line $i$;
- $t_{oc}$ is the time of door opening and closing (s);
  - $a$ is passenger boarding time (s/p);
  - $b$ is passenger alighting time (s/p);
  - $\theta$ is the transfer penalty;
- $x_j$ is the number of buses stopping at stop $j$ with skip-
  stop operation;
- $B_j$ is the capacity of stop $j$, from Table 1 [1].

### Table 1: Capacity of on-line linear stops (buses/h).

<table>
<thead>
<tr>
<th>Number of loading areas</th>
<th>Dwell time (s)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>15</td>
</tr>
<tr>
<td>1</td>
<td>63</td>
</tr>
<tr>
<td>2</td>
<td>117</td>
</tr>
<tr>
<td>3</td>
<td>154</td>
</tr>
</tbody>
</table>

Assuming 15 s clearance time, 25 percent queue probability, 60 percent
coefficient of variation of dwell times, and 0.5 g/C.

2. Travel Time Model under Mixed
Traffic Conditions

The travel time depends on the traffic volume and capacity of
links, which is influenced by the number of stopping buses
at a stop station under mixed traffic conditions. The BPR
(Bureau of Public Roads) function, the most classic model,
describes the link performance, which states the relationship
between resistance and traffic volume [10]. In this section, the
impact of bus stops on the link capacity is analyzed, and then
the model of computing travel time is established based on
the data of simulation runs.

2.1. Link Capacity. Curbside bus stops interfere with traffic
flows, as the buses stop in the travel lane, resulting in a
"bottleneck" (the reduction in the road width) at the
location of the stops. Bus bays interfere with passing vehicles
primarily, while buses maneuver to pull into and out of
the stops. In general, the capacity is calculated by adding adjustment factor for bus stops [1], as follows:

\[ C_t = C_0 \cdot (1 - f_s) = C_0 \cdot \left(1 - \frac{T_e}{3600}\right). \quad (1) \]

When all passing buses stop at this stop, the impacting time is determined by the numbers of loading areas and passengers getting on and off buses. If the bus stop is considered as a queuing system, the probability of no bus being serviced can be calculated by the following equation [11]:

\[ p = \left[ \sum_{k=0}^{S-1} \frac{(\lambda \tau)^k}{k!} + \frac{(\lambda \tau)^S}{S! (1 - \lambda \tau / S)} \right]^{-1}. \quad (2) \]

So the impacting time by bus stops is calculated by

\[ T_e = 3600 \cdot (1 - p). \quad (3) \]

When skip-stop service is adopted, there are two possible scenarios at stops in a skip-stop operation: (1) There is no bus being serviced when skip-stopped bus \( N \) arrives at this stop. (2) There is one or more buses being serviced when skip-stopped bus \( N \) arrives at this stop. In the first scenario, the bus passes directly through with no impact on the capacity of the adjacent lane. In the second scenario, the bus would pass buses being serviced on the curb lane. The lane changing behavior will affect the operation of the vehicle in the adjacent lane and result in the dropping of capacity. According to HCM2000, the affecting time is 4 seconds when one bus uses the adjacent lane.

The probability of the first scenario is determined by the number of buses stopped at this stop, which is computed by the following equation:

\[ p_1 = \left[ \sum_{k=0}^{S-1} \frac{(\lambda_1 \tau)^k}{k!} + \frac{(\lambda_1 \tau)^S}{S! (1 - \lambda_1 \tau / S)} \right]^{-1}. \quad (4) \]

The impacting time in a skip-stop operation is approximately calculated using

\[ T'_e = 3600 \cdot (1 - p_1) + 4 \times 3600 (\lambda - \lambda_1) (1 - p_1) \]
\[ = 3600 (1 + 4 (\lambda - \lambda_1)) \]
\[ \times \left(1 - \left[ \sum_{k=0}^{S-1} \frac{(\lambda_1 \tau)^k}{k!} + \frac{(\lambda_1 \tau)^S}{S! (1 - (\lambda_1 \tau) / S)} \right]^{-1}\right). \quad (5) \]

Therefore, the capacity of links influenced by bus stops is calculated by

\[ C_t = C_0 \cdot \left(N - \frac{T_e}{3600}\right) \]
\[ = C_0 \left(N - (1 + 4 (\lambda - \lambda_1)) \times \left(1 - \left[ \sum_{k=0}^{S-1} \frac{(\lambda \tau)^k}{k!} + \frac{(\lambda \tau)^S}{S! (1 - (\lambda \tau) / S)} \right]^{-1}\right)\right). \quad (6) \]

The comparison results of different types of vehicles are shown in Table 2.

A paired \( t \)-test of null hypothesis of no-mean difference is performed to check for the match between simulated and observed average speed of values vehicles. The calculated value of \( t \) is 0.6095 against the critical values (from “\( t \)” table) of 1.648. It is found that the observed and simulated average speed values agree at 5% level of significance (95% confidence limit). It can be seen that the simulated speed values significantly replicate the observed field speed for all vehicle types, and the VISSIM model can be applied to study the characteristics of traffic flow on urban road links.

2.2. Simulation Model Development and Validation. The classic BPR function is calibrated using the traffic data of highway. Therefore, the parameters need to be recalibrated when BPR function is used to describe the performance of urban roads. It is difficult to survey traffic operating data under various conditions. So the parameters of BPR function are calibrated using the simulated data in this paper.

Based on the paper’s objectives as well as the required details in the analysis, VISSIM (version 4.2) is employed in this paper. The simulation model is developed using VISSIM based on the surveyed data. For the purpose of model validation, the mean speed of values cars and buses observed on the field and simulated by VISSIM are compared. The comparison results of different types of vehicles are shown in Table 2.

A paired \( t \)-test of null hypothesis of no-mean difference is performed to check for the match between simulated and observed average speed of values vehicles.

\[ T(q) = T_0 \left[ 1 + \alpha \left(\frac{q}{C_j}\right)^\beta\right]. \quad (7) \]
In order to improve the reliability of VISSIM model, the simulation runs are made with random number seeds ranging from 41 to 45, and the average of the five values is taken as the final model output. The travel time outputted from the model is as shown in Figure 1.

In this paper, BPR model is calibrated by the method of Least-squares. First, BPR model should be deformed as a linear function by logarithmic transformation \[12\].

\[
\ln \left( \frac{V_0}{V} - 1 \right) = \ln \alpha + \beta \ln \left( \frac{q}{C_l} \right). \tag{8}
\]

Denoting \(\ln(q/c)\) by \(X\), \(\ln(V_0/V - 1)\) by \(Y\), \(\beta\) by \(A\), and \(\ln \alpha\) by \(B\), then (8) is transformed to

\[Y = AX + B. \tag{9}\]

Second, these above data are transformed to \(X\) and \(Y\). Third, the values of \(A\) and \(B\) are estimated by the method of least-squares; then, the values \(\alpha\) and \(\beta\) are 4.2 and 0.87, respectively. Therefore, the travel time of cars is obtained when there is an on-line curbside bus stop on the road, as shown in the following:

\[T(q) = T_0 \left[ 1 + 0.87 \left( \frac{q}{C_l} \right)^{4.2} \right]. \tag{10}\]

3. Skip-Stop Design Model Formulation

As indicated in the previous studies, the skip-stop problem can be formulated as a nonlinear 0-1 integer programming problem, with the binary integer variables representing which stops to be skipped by the control vehicles. In this study, for the purpose of solving the exact problem, the stop-skipping problem will be formulated again as a nonlinear 0-1 integer programming problem.

3.1. Assumptions. In order to analyze the process, the following assumptions are made in this study:

1. The total travel demand between any origin and destination node pair (O-D matrix) is fixed and remains the same during the analysis period. It is assumed that the travel demand is not affected by the introduction of skip-stop operation.
2. There are only two modes of traffic (buses and cars) utilizing the network.
3. Dwell time at each stop is determined by the number of passengers getting on and off the bus.
4. Passengers are uniformly distributed throughout the area.
5. A passenger would not leave the platform, and she or he would not wait more than two times.

3.2. Skip-Stop Design Models under Mixed Traffic Conditions. Run time models are usually used in understanding the existing service and evaluating several transit planning and
Table 3: Traffic data on Tian-Mu-Shan Road.

<table>
<thead>
<tr>
<th>No.</th>
<th>Bus stop</th>
<th>Car volume (veh/h)</th>
<th>Distance between adjacent stops (m)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>Gucui Intersection</td>
<td>930</td>
<td>—</td>
</tr>
<tr>
<td>2</td>
<td>Xueyuan Intersection</td>
<td>975</td>
<td>280</td>
</tr>
<tr>
<td>3</td>
<td>Qingfeng Village, West</td>
<td>893</td>
<td>522</td>
</tr>
<tr>
<td>4</td>
<td>Qingfeng Village, East</td>
<td>920</td>
<td>395</td>
</tr>
<tr>
<td>5</td>
<td>Xixi District</td>
<td>823</td>
<td>345</td>
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<tr>
<td>6</td>
<td>Bazi Bridge</td>
<td>760</td>
<td>550</td>
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<tr>
<td>7</td>
<td>Macheng Intersection</td>
<td>726</td>
<td>270</td>
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<td>8</td>
<td>City Government</td>
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<td>Hushu Intersection</td>
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<td>Hangzhou Building</td>
<td>770</td>
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<td>11</td>
<td>Zhongshan Intersection</td>
<td>780</td>
<td>410</td>
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</table>

4. Optimization Process Using Genetic Algorithm

The proposed model is a nonlinear programming problem which associates the zero-one variables, and the parameters are tightly related to each other. It is hardly solved with conventional solution methods. The genetic algorithm (GA) is a heuristic search method that imitates the process of natural evolution [15]. It is motivated by the principles of
Figure 4: The diagram of skip-stop operation scenario. Note that “∙” represents stopping, “○” represents skipping, and “⊗” represents not passing by.

Table 4: Numbers of passengers boarding and alighting at stops (from west to east).

<table>
<thead>
<tr>
<th>Line</th>
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<th>11</th>
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<tbody>
<tr>
<td>1</td>
<td>19/14</td>
<td>14/45</td>
<td>24/41</td>
<td>27/63</td>
<td>42/56</td>
<td>36/33</td>
<td>16/22</td>
<td>49/65</td>
<td>21/34</td>
<td>28/58</td>
<td>16/22</td>
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<td>18/57</td>
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Table 5: The optimal scenario of skip-stop operation.

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Note that "1" represents stopping, "0" represents skipping, and "—" represents not passing by.

Table 6: The comparison between all-stop and skip-stop operations.

<table>
<thead>
<tr>
<th>Scenario</th>
<th>Car travelers</th>
<th>Bus travelers</th>
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<tbody>
<tr>
<td></td>
<td>Travel time (min)</td>
<td>Out-of-vehicle time (min)</td>
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<tr>
<td>Original scenario</td>
<td>34,966</td>
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<td>Optimal scenario</td>
<td>30,471</td>
<td>12,228</td>
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<tr>
<td>Change</td>
<td>−12.86%</td>
<td>43.77%</td>
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</table>

natural selection and survival of the fittest individuals. This method is commonly used to generate useful solutions to optimization problems [16]. Therefore, the genetic algorithm is used to solve the bilevel programming model and find the optimal coordination of the stopping stations in order to minimize inconveniences for passengers. Based on the above analysis, the procedure for finding an optimal scenario is presented, as shown in Figure 2.

For the genetic algorithm, each gene location in a chromosome represents a possible skip-stop choice, while the vehicles of bus line traverse every station shown in Figure 3, where “1” indicates stops and “0” indicates skips at the corresponding station. In Figure 3, a chromosome is divided into $I$ independent parts. Each part indicates the corresponding operation choices for every bus line.

5. Case Study

5.1. Traffic Survey. This paper takes Tian-Mu-Shan Road (from Wan-Tang Road to Zhong-Shan Road) as an example, which is a heavy-demand corridor of Hangzhou City. The examined period is the morning peak. The traffic volume and the number of passengers getting on and off at 11 stops are collected, as shown in Tables 3 and 4.

5.2. Results. Parameters values of the model and the algorithm are shown as follows: the population of chromosomes (individuals) for each generation $N = 50$, the crossover probability $P_c = 0.5$, the mutation probability $P_m = 0.01$, the biggest generation $GenMax = 200$, $a = b = 1 \text{ m/s}^2$, and $L = 500 \text{ m}$. The optimization process is programmed using MATLAB software. Then, these data of Tables 3 and 4 are input to program, and an optimal scenario is outputted, as shown in Figure 4 and Table 5.

The travel times of all travelers before and after the optimization are compared, as shown in Table 6. It can be seen that the travel time of car travelers becomes 12.86 percent shorter than that with original all-stop operation, and the travel time of bus travelers reduces 6.36 percent due to skipping stations at the same time. The total travel time of all travelers becomes 7.03 percent shorter after the implementation of skip-stop operation. Results show that the optimization scheme can improve the overall operating efficiency of road network.
6. Conclusions
The flow capacity is adjusted at the section of stop zone based on the analysis of the mutual influence between buses and cars at bus stops. BPR function including the modified capacity is calibrated using traffic simulation. Then, this paper proposes an optimization model for designing skip-stop service that can minimize the total travel time of passengers under mixed traffic conditions. The genetic algorithm is used to find the optimal coordination of the stopping stations of overall bus lines in an urban bus corridor. The validation of the model and the algorithm have been proved with the help of a real-world case. Results show that the operational efficiency of buses and cars is improved with skip-stop operation under mixed traffic conditions. Although skip-stop operation is complicated for the operators and it can confuse passengers at the beginning of the service, it can certainly reduce passengers’ total travel time. It should be noted that the dwell time of a bus is assumed to be based on the number of passengers boarding and alighting, as a matter of fact, which is also influenced by the number of stopping buses at the stop. Therefore, an interesting project for future research would be to propose an optimization method for designing skip-stop services with the consideration of the bus operating features at stops.

Conflict of Interests
The authors declare that there is no conflict of interests regarding the publication of this paper.

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References