Research Article

Strength and Failure Characteristics of Natural and Water-Saturated Coal Specimens under Static and Dynamic Loads

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Rock bursts occur frequently in coal mines, and the mechanical properties of saturated coal specimens under coupled static-dynamic loading need to be studied in detail. Comparative tests of coal specimens having different water content under static and static-dynamic loading are conducted using the split Hopkinson pressure bar (SHPB) and RMT-150C test systems. The results demonstrate that the natural specimen strength is greater than that of seven-day (7D) saturated specimens under both uniaxial compression and triaxial static compression loading; however, the dynamic strength of 7D saturated specimens is lower than that of natural specimens under one-dimensional static-dynamic loading. The particle size of the 7D saturated specimens is relatively small under uniaxial static compression and one-dimensional static-dynamic loading, and the specimen particle sizes before and after static triaxial loading tests and three-dimensional static-dynamic loading tests do not exhibit an obvious difference.

1. Introduction

Geotechnical engineering practices, such as mining, slope stability, and water conservation, are significantly affected by water. Rocks and rock masses are multiphase composite natural mediums including a large number of pores and cracks with different scales, thereby containing a certain amount of water. In general, under saturated conditions, the presence of water has a significant effect on the physical, chemical, and mechanical properties of coal and rocks.

Microseismic monitoring can play an important role in the prediction of the rock bursts in deep mines [1–3]. As the moisture content of coal increases or decreases, the characteristics of microseismic and acoustic emissions also change. Few studies exist on the strength and failure characteristics of water-saturated coal specimens under impact, and the results of such an investigation are helpful in preventing rock burst.

The mechanical properties (namely, uniaxial compressive, tensile, and shear strengths) of saturated and dry rocks have been studied extensively under static loading. Research results demonstrate a trend whereby the quasistatic rock

strength decreases following saturation [4–10]. However, the dynamic rock strength following saturation exhibits uncertainty in terms of increase and decrease. Thus far, numerous research studies have been conducted on the mechanical properties of rock under dynamic loading at different strain rates [11]. Huang et al. [12] studied the mechanical properties of saturated sandstone tensile strength under dynamic loading in SHPB system, and the results indicated that the tensile strength softening factor decreases with the loading rate. Yan and Ma [13] conducted uniaxial dynamic compression tests on sandstone under different saturation conditions, demonstrating that the dynamic strength of saturated sandstone increased by 18 to 29% compared to dry sandstone. Moreover, static and dynamic loading strength tests of sandstone under different water bearing conditions were carried out in SHPB system by Zhou et al. [14, 15], revealing that the static and dynamic sandstone strengths decreased by 29.88% and 40.55%, respectively, following saturation.

Rock bursts frequently occur around roadways and working faces in coal mines [16–19]. During this process, a coal seam exists in the stress environment under medium strain

FIGURE 1: Rock burst induced by breaking of hard roof structure.

rate loading, and the coal seam failure degree can be directly affected by water content. Water injection may alleviate or prevent the occurrence of rock bursts [20, 21]. Numerous research studies have been carried out on the mechanical properties of coal specimens under static loading: Su et al. [22, 23] studied the effects of saturation time on the coal burst tendency index; Pan et al. [24] conducted research on the saturated time effect in the Qianqiu coal mine as well as the coal seam water injection method; and Liu et al. [25, 26] investigated the mechanical properties of water-injected coal specimens by using water injection test equipment. However, the dynamic mechanical properties of saturated coal specimens have rarely been studied.

Rock bursts with static and dynamic coupling loading frequently occur in the Yima coal mining area, Henan province, China. The thick (80~400 m) and hard rock roof may be broken, as shown in Figure 1, generating frequent impact loadings on coal seam under a preexisting high static load and resulting in the occurrence of rock bursts. In order to alleviate coal seam rock burst, the water injection method is used to increase the coal water content, which weakens the coal seam and thus reduces the possibility of rock burst. Therefore, the strength characteristics and damage morphology of natural and seven-day (7D) saturated specimens under both a 1D and 3D static load (static-dynamic load loading) are studied. The results provide theoretical and engineering guidance for rock burst prevention in coal mines.

2. Experimental Work

2.1. Specimen Preparation. Coal specimens were collected from the number 21 coal seam in the Yuejin coal mine. In order to reduce data scattering in the tests, the specimen was cut from the same working face, as shown in Figure 2. The main component of the number 21 coal seam is long flame coal, with a W^f of 4.88%, H_m^f of 4.25%, and Rom of 0.505. The specimens were $\phi 50 \text{ mm} \times 30 \text{ mm}$ cylinders with nonparallelism of less than 0.05 mm and surface evenness of less than 0.02 mm, in accordance with standards in

the International Society for Rock Mechanics (ISRM) [27], suggested specification, as illustrated in Figure 3.

With porosity cracks and a multijoint structure inside, coal is a complicated and heterogeneous type of rock, differing from limestone, granite, and sandstone on both the macroscopic and microcosmic scale. When enlarged 200 to 500 times using the scanning electron microscope (SEM), as shown in Figure 4, the coal specimens exhibited obvious voids and clear cracks with a large opening degree, while few cracks were present in the limestone. During the test, the selected specimens were divided into two groups: coal in its natural state and that in 7D saturation (saturated in water for 168 h). The water content of the specimens ranged from 3.2 to 6.1%, and the porosity and the coefficient permeability were 5.83% and $0.3541 \sim 0.3849 \text{ m}^2/(\text{Mpa}^2 \cdot \text{d})$, respectively.

To enable comparison, the specimen sizes used in the static and static-dynamic coupling tests were the same. Specimens numbered from A1 to A6, B1 to B6, C1 to C10, and D1 to D10 were used in the static uniaxial compression test, 1D static-dynamic loading, true triaxial static compression test, and 3D static-dynamic loading, respectively.

2.2. Testing Facilities. Static tests were conducted using the RMT-150C rock mechanism experimental system at Henan Polytechnic University, China, as shown in Figures 5(a) and 5(b). The testing frame stiffness was $5.0 \times 10^9 \text{ N/mm}$; the strain rates covered in the tests ranged from 10^{-1} to 10^{-6} s^{-1} , and the maximum load was 1000 kN. The test range was 50 mm, the maximum confining pressure was 50 MPa, and the confining pressure rate ranged from 0.001 to 1 MPa/s. This system can characterize dynamic responses and obtain stress-strain curves for uniaxial and triaxial compression tests using different control methods.

Experiments under static-dynamic loading were performed using SHPB system at Central South University, China, as shown in Figures 5(c) and 5(d). The SHPB system consisted of two bars made of 40Cr alloy; the diameter of the bars was 50 mm and the elastic compression wave velocity was 5410 m/s. This system can be used for testing the

FIGURE 2: Location of Yuejin coal mine: (a) Yuejin coal mine in Henan province, China, and (b) selected specimen locations in 25080 working face.

FIGURE 3: Specimens for static and dynamic testing: (a) specimens before processing and (b) specimens after processing.

FIGURE 4: SEM testing photos of coal and Shendong limestone specimens.

FIGURE 5: Various parts of RMT-150C and SHPB testing system: (a) uniaxial static system; (b) triaxial static system; (c) SHPB testing system; and (d) triaxial static-dynamic system.

FIGURE 6: Schematic of SHPB testing system.

FIGURE 7: Mechanics diagrams of specimen under static and dynamic coupling loads.

dynamic properties of heterogeneous brittle materials, such as rock, concrete, and coal seams, under different confinement conditions. The axial precompression ranged from 0 to 200 MPa the confining pressure ranged from 0 to 20 MPa, and the dynamic impact load ranged from 0 to 500 MPa. The system employs half-sine wave loading in order to reduce the waveform dispersion and achieve a constant deformation rate by using a special-shaped bullet. Further details can be found in [28].

2.3. Experimental Plans. During static compression tests, the axial deformation and the load were measured by a 5 mm displacement transducer and 1000 kN compression transducer, respectively. During the experiment, axial loading was applied by means of displacement control with a loading speed of 0.002 mm/s. In the triaxial confining compression tests, the confining pressures were set as 2, 4, 6, 8, and 10 MPa. The natural and 7D saturated specimens were tested under uniaxial and triaxial static compression conditions, respectively.

For the static-dynamic coupling tests, 1D static-dynamic tests were conducted, where a static load of 12 MPa was firstly imposed on the specimen prior to dynamic loading, following which dynamic loading was applied by a bullet impacting the end of the incident bar. The bullet was driven by a gas gun having 0.4 MPa gas pressure. In this manner, the strength and

failure characteristics of natural and 7D saturated specimens were investigated. In the 3D static-dynamic loading tests, a confining compression of 4 MPa and axial compression of 8, 12, 16, 20(28), and 36 MPa were applied to the natural and 7D saturated specimens in order to conduct group tests. A schematic of the SHPB testing system is provided in Figure 6. The specimen stress states under different loadings are shown in Figure 7, where (a) illustrates the specimen loading process under static uniaxial compression; (b) shows the 1D static-dynamic loading process; (c) illustrates the loading process of specimens under triaxial static compression; and (d) shows the loading process of specimens under 3D static-dynamic loading.

The SHPB tests were based on the assumption that the 1D elastic wave theory is applicable during stress wave propagation in the long bar. The stress, strain, and strain rate of the specimens were measured by strain gauges on the pressure bars, which can be determined by

$$\begin{aligned}\sigma(t) &= \frac{A_e E_e}{2A_s} [\varepsilon_I(t) + \varepsilon_R(t) + \varepsilon_T(t)], \\ \varepsilon(t) &= \frac{C_e}{L_s} \int_0^t (\varepsilon_I(t) - \varepsilon_R(t) - \varepsilon_T(t)) dt, \\ \dot{\varepsilon}(t) &= \frac{C_e}{L_s} [\dot{\varepsilon}_I(t) - \dot{\varepsilon}_R(t) - \dot{\varepsilon}_T(t)],\end{aligned}\quad (1)$$

FIGURE 8: Stress-strain curves of natural and saturated coal samples under static loads.

where A_s and A_e are cross-sectional areas of the specimens and pressure bar, respectively; E_e is the elastic modulus of the pressure bar; ε_I , ε_R , and ε_T denote the incident, reflected, and transmitted strain waves measured on the incident and transmission bars, respectively; and C_e and l_e are the stress wave velocity in the bar and tested specimen length, respectively.

3. Results and Discussion

3.1. Results of 1D Static Tests. Stress-strain curves for the natural and 7D saturated specimens under static uniaxial compression are depicted in Figure 8. The uniaxial compression strength of natural coal specimens ranges from 42.07 to 43.11 MPa, with an average of 42.71 MPa, as shown in Table 1. The uniaxial compression strength for the 7D saturated coal specimens ranges from 20.40 to 25.30 MPa, with an average of 22.17 MPa. The softening coefficient of the uniaxial compression strength between the 7D saturated and natural state ranges from 0.49 to 0.59, and the average is 0.52. The coal strengths exhibit certain scatterings, and the coal compression strength and elastic modulus are affected by 7D saturation to a certain degree, indicating a trend of decreasing strength with increasing saturation time. In general, the coefficient of rock is between 0.7 and 0.95. As coal is sedimentary with natural multicracks, these cracks are more developed than in other rocks; therefore, the coal peak softening coefficient is higher than that of rocks.

The specimen fissure development during static compression tests can be divided into four stages, as follows: (I) Compaction stage: the microcracks are compacted, and the stress-strain curve exhibits an upward concave shape during this stage. (II) Elastic stage: the characteristics of this stage are that the microcracks continue to compact and close, while the stress-strain curve exhibits a linear increase. (III) Expansion and failure stage: primary cracks develop, and secondary cracks are induced, and the stress-strain curve exhibits a fluctuation, declining and reaching the peak value. (IV) Peak

failure stage: slip failure of the specimen appears during this stage.

3.2. Results of 1D Static-Dynamic Loading Tests. 1D static-dynamic loading tests were conducted on natural and 7D saturated specimens, with four specimens used in each state. The dynamic strain rate ranges from 90 to 155 s^{-1} . Figure 9 illustrates the specimens in different saturated states.

The dynamic compression strengths of the natural specimens range from 43.47 to 46.93 MPa, with an average of 46.71 MPa, as shown in Table 2. The dynamic compression strengths of the 7D saturated specimens range from 13.50 to 40.63 MPa, with an average of 28.20 MPa. The average softening coefficient of dynamic strength between the natural and 7D saturated specimens is 0.60. The dynamic compression strength exhibits a decreasing tendency with an increase in water content. A large difference exists between the natural and 7D saturated specimen dynamic compression strengths, and the specimen strengths in the same saturated state also exhibit scattering, indicating that coal specimens possess heterogeneous characteristics.

The compression strength of the coal specimen under both static and 1D static-dynamic loadings decreases with an increase in saturation time. Although the stress-strain curves of the natural and 7D saturated specimens under 1D static-dynamic tests exhibit a similar trend, certain differences remain in the elastic stage. The yield strength of the static-dynamic loading tests is close to that in the static loading tests. The strength of specimens under static-dynamic loading conditions is increased by 10 to 30% compared to that under static loading.

Experimental results from Wang et al. [29] demonstrate that, at a strain rate of 52–56 s^{-1} , the saturated sandstone stress-strain curve is similar to that of natural sandstone in the uniaxial dynamic test; however, the saturated sandstone dynamic strength is higher than that of natural sandstone, which is opposite to the tendency of the saturated sandstone dynamic strength decreasing with increasing water content.

TABLE 1: Natural and saturated specimen parameters under uniaxial static compressive tests.

Specimen number	Saturated status	Diameter/mm	Length/mm	Density/g·cm ⁻³		Strength/MPa		Failure strain/%		Elastic modulus/GPa	
				Dry	Wet	Tested	Average	Tested	Average	Tested	Average
A1	Natural	4.97	2.86	1.41	/	43.11	42.71	28.57	25.64	2.62	2.65
A2	Natural	4.98	2.95	1.39	/	42.95		22.79		2.74	
A3	Natural	3.98	3.02	1.41	/	42.07		25.57		2.59	
A4	7D water	4.97	3.00	1.41	1.47	25.30	22.17	20.53	23.01	1.28	1.77
A5	7D water	4.98	3.05	1.38	1.46	20.81		32.71		2.11	
A6	7D water	4.98	3.01	1.37	1.45	20.40		15.78		1.92	

TABLE 2: Natural and saturated specimen parameters under uniaxial static-dynamic compressive tests.

Specimen number	Saturated status	Diameter/mm	Length/mm	Density g/cm ⁻³		Static uniaxial pressure/MPa	Strain rate /s ⁻¹	Strength/MPa		Grain size/mm
				Dry	Wet			Tested	Average	
B1	Natural	4.95	3.06	1.34	/	12	107	46.93	46.72	17.09
B2	Natural	4.98	3.09	1.33	/	12	131	46.50		16.42
B3	Natural	4.98	3.04	1.48	/	12	117	43.47		15.84
B4	Natural	4.96	3.11	1.30	/	12	111	49.96		16.11
B5	7D water	4.96	3.04	1.35	1.41	12	115	17.73	28.44	13.18
B6	7D water	4.96	2.98	1.41	1.48	12	128	40.63		12.08
B7	7D water	4.99	3.00	1.36	1.42	12	97	31.50		12.01
B8	7D water	4.95	3.14	1.34	1.39	12	128	23.88		13.54

FIGURE 9: Stress-strain curves of natural and 7D saturated specimens in uniaxial static-dynamic loads.

FIGURE 10: Static stress-strain curves of natural and 7D saturated specimens under triaxial compression.

The saturated sandstone dynamic strength is twice as high as the static strength, with a greater improvement than that of coal specimens.

3.3. Results of 3D Static Loading Tests. As illustrated in Figure 10, the static strength of natural specimens under triaxial loading ranges from 76.39 to 84.41 MPa, with an average of 81.01 MPa and fluctuation of 10.49%. The static strength of 7D saturated specimens ranges from 49.39 to 77.54 MPa, with an average of 65.54 MPa and fluctuation of 59.98%, as shown in Table 3. The softening coefficient of 7D saturated specimens compared to natural specimens is 0.81.

The deformation behavior of the natural state and 7D saturated specimen stress-strain curves are roughly the same before the peak value, exhibiting impaction, elastic, and yield stages. However, distinct differences are apparent following the peak. The postpeak stress of natural specimens obviously decreases and shows the features of brittle failure, while

the postpeak stress of the 7D saturated specimens decreases slowly and exhibits plastic features. As coal contains multivoids and multicracks, under triaxial static loading, the coal strength and deformation behavior are affected by saturation conditions to differing extents, and water has an obvious softening effect on cracked coal specimens.

3.4. Results of 3D Static-Dynamic Loading Tests. Under the 3D loading system with the same dynamic loading and confining pressure, the dynamic strength of the natural and 7D saturated specimens under different axial pressures are obtained when the confining pressure is 4 MPa, as shown in Table 4. The dynamic strength of the natural specimens under different axial pressures (8 to 36 MPa) varied from 54.23 to 70.16 MPa, as shown in Figure 11. The results demonstrate that the dynamic strength of 20 MPa under axial compression is 39.02% higher than that of axial compression at 8 MPa. The dynamic strength of the 7D saturated specimens under

TABLE 3: Natural and water saturated specimen parameters under triaxial static compressive tests.

Specimen number	Saturated status	Diameter/mm	Length/mm	Density $\text{g}\cdot\text{cm}^{-3}$		Confining pressure /MPa	Strength/MPa		Failure strain/%		Elastic modulus/GPa	
				Dry	Wet		Tested	Average	Tested	Average	Tested	Average
C1	Natural	4.98	3.01	1.36	/	2.0	76.39	80.42	2.26	2.45	3.22	3.34
C2	Natural	4.98	2.98	1.43	/	4.0	83.83		2.51		3.55	
C3	Natural	4.98	2.94	1.36	/	6.0	78.09		2.44		3.21	
C4	Natural	4.99	3.02	1.41	/	8.0	79.39		2.49		3.25	
C5	Natural	4.99	3.18	1.40	/	10.0	84.41		2.57		3.54	
C6	7D water	4.98	2.98	1.43	1.51	2.0	69.35	65.54	3.22	2.84	2.18	2.47
C7	7D water	4.98	3.11	1.41	1.48	4.0	77.54		3.29		2.54	
C8	7D water	4.98	2.93	1.38	1.45	6.0	56.19		2.37		2.61	
C9	7D water	4.99	2.96	1.35	1.42	8.0	49.39		2.96		1.78	
C10	7D water	4.98	3.00	1.38	1.46	10.0	75.21		2.37		3.22	

TABLE 4: Natural and saturated specimen parameters under triaxial static-dynamic compressive tests.

Specimen number	Saturated status	Diameter/mm	Length/mm	Density/ g·cm ⁻³		Confining pressure/ MPa	Axial pressure /MPa	Strain rate /s ⁻¹	Strength/MPa		Failure strain/%	
				Dry	Wet				Tested	Average	Tested	Average
D1	Natural	4.97	3.04	1.42	/	/	8	76	54.23	66.63	1.07	0.922
D2	Natural	4.98	2.96	1.42	/	/	12	59	65.07		0.91	
D3	Natural	4.97	2.93	1.43	/	4 MPa	16	59	68.27		0.98	
D4	Natural	4.98	2.99	1.44	/	/	20	60	75.39		0.98	
D5	Natural	4.97	3.13	1.41	/	/	36	63	70.16		0.67	
D6	7D water	4.97	3.06	1.44	1.51	/	8	68	63.21	74.55	1.22	1.134
D7	7D water	4.98	3.07	1.44	1.52	/	12	68	70.29		1.17	
D8	7D water	4.98	3.06	1.41	1.49	4 MPa	16	81	80.86		1.12	
D9	7D water	4.98	3.07	1.40	1.48	/	28	70	82.71		1.02	
D10	7D water	4.97	3.02	1.44	1.49	/	36	67	75.67		1.14

FIGURE 11: Dynamic stress-strain curves of natural and 7D saturated specimens under confining pressure of 4 MPa.

different axial pressures (8 to 36 MPa) was 63.21 to 75.67 MPa. The 7D saturated specimen test results indicate that the dynamic strength of axial pressure at 28 MPa is 30.85% higher than that at 8 MPa. During the elastic stage, it appears that the specimens subjected to larger axial pressure exhibit greater dynamic strength under the 3D stress condition. Furthermore, the influence of axial pressure on the dynamic strength of the 7D saturated specimens is similar to that of the natural specimens.

Under different axial pressure conditions, the dynamic strength of the specimens first increases and then decreases with an increase in axial pressure. The turning points of the specimen dynamic strengths occur when axial pressure is 20 and 28 MPa (47% and 66% of the uniaxial static strength, resp.), and the dynamic strength of the natural and 7D saturated specimens exhibits a decreasing characteristic during the test. The dynamic strength of the saturated 7D specimens is higher than that of the natural specimens by 7.85% to 18.44%, which indicates that the dynamic strength of 7D saturated specimens increases compared to that of natural specimens in the 3D static-dynamic tests.

As a porous medium, coal often has a lower compactness than ordinary sandstone and limestone, and numerous irregular fissures and pore exist in the coal seam. When the coal specimen is loaded under axial compression and confining pressure, it remains in the elastic state under 3D compression stress, and it becomes smaller in volume as the fissures and pores are gradually compressed. When the axial pressure increases and the specimen remains in the elastic range, the specimen dynamic strength also increases. The effects of axial compression and confining pressure on the specimen dynamic strength manifest mainly in the elastic range; when the axial pressure increases to the point where specimens show swelling damage, the internal cracks of the coal increase gradually and the dynamic strength decreases.

4. Broken Characteristics of Natural and Saturated Specimens

4.1. Broken Characteristics of Specimens in 1D Static-Dynamic Loading. To monitor the failure process of specimens under dynamic loading, a high-speed camera (FASTCAM SA1.1) was employed to capture coal specimens during the SHPB test. Under the condition of 1024×1024 resolution, the camera can achieve a frame rate of 5400 fps, and it can be as high as 675000 fps at the lowest resolution. In this experiment, the frame rate was 100000 fps, with pixels at 192×192 resolution, and the time space between two adjacent captured photos was approximately $10 \mu\text{s}$. In the 1D static-dynamic loading tests, both the natural and 7D saturated specimens were photographed, based on which the surface crack propagation characteristics of the specimens were analyzed, and the dynamic failure process of the natural and 7D saturated specimens is discussed in detail.

Figures 12 and 13 illustrate the fracture propagation and failure process of the natural specimen (B2) and 7D saturated specimen (B8), respectively.

When the natural specimen (B2) is impacted in the uniaxial dynamic test, a single crack appears on it at the 200th μs . At the 400th and 600th μs , the cracks increase significantly and penetrate the entire specimen. At the 800th μs , the number of cracks and the crack width gradually increases, accompanied by the appearance of small secondary cracks. The specimen fracture width gradually increases at the 1000th μs , and at the 1200th μs , the integrity of the specimens changes, and they lose bearing capacity as particles begin to burst out. The period of the specimen fracture from first damaged to total failure lasted for 1200th μs , and the specimen fragmentation degree is relatively large.

The 7D saturated specimen (B8) appears obvious cracks at 100th μs . Although numerous cracks penetrate the specimens at the 300th μs , the specimen remains intact; more cracks

FIGURE 12: High-speed photos of natural specimen (B2) in dynamic failure processing.

FIGURE 13: High-speed photos of 7D saturated specimen (B8) in dynamic failure processing.

appear at the 500th μ s, at which point the cracks coalesce with one another. The crack width at areas A and B increases gradually, along with serious damage occurring. At the 700th μ s, relatively wide cracks appear in areas A and B, and large coal particles are ejected, with specimen integrity being destroyed. At the 900th and 1100th μ s, large numbers of particles are ejected and the fragments are relatively small.

Comparing the dynamic failure process of the natural and 7D saturated specimens, the failure duration of the natural specimens is longer, the fragment is larger, and the specimen integrity is maintained for longer, while the destruction duration of the 7D saturated specimens appears shorter and the destroyed particles are smaller. The reason for this phenomenon is that the coal specimens contain numerous voids, and the voids in the natural specimens contain air, while those in the 7D saturated specimens contain water. The failure process of the natural and saturated 7D specimens was statistically analyzed, demonstrating the failure characteristics of the natural (B2) and 7D saturated (B8) specimens. The water content of the 7D saturated specimen increases and both the voids and cracks contain water. When the static load is applied, the specimen is in the elastic state, and the pores and fractures are compressed. In this case, the fracture is under the compression of the void water pressure.

Under the action of dynamic loading, the stress wave propagates onto the fracture and the water contained therein, and the pore compression deformation is equivalent to the pore water pressure acting on the fracture wall; thus, the failure time of the 7D saturated specimen shortens. Moreover, the saturated water has a weakening effect on the fracture bond, resulting in a decrease in the fracture cohesive force. Therefore, compared to static loading, coal specimen cracks

appear to initiate and propagate more easily under the action of a stress wave, and the particle breakage size is smaller.

4.2. Destroyed Particle Statistics of Natural and 7D Saturated Specimens. The particle masses following impact were statistically analyzed. The particles were classified into three grades, according to fragment sizes of less than 30, 10, and 5 mm. The particles were screened, and the quality of each grade and total quality percentage (%) were calculated as shown in Table 5.

(1) In the static uniaxial compression test for the natural and 7D saturated specimens, the natural specimen integrity following failure is more obvious, and the average sizes of particles less than 30 mm range from 26 to 33% of the total mass in Figure 14. However, the failure degree of the 7D water-saturated specimens became more serious following impact, and the percentage of particles with diameters less than 30 mm ranges from 39 to 74% of the total mass.

(2) While testing the natural and 7D saturated specimens under 1D static-dynamic loading, the average size of the natural specimen broken particles is larger than that of the 7D saturated specimen, as shown in Figure 15. For the natural coal specimens, the sizes of particles of less than 30 mm range from 69.6 to 82.5% of the total mass; for 7D saturated specimens, the percentage ranges from 83.8 to 94.1% of the total mass.

(3) In the triaxial static compression test and 3D static-dynamic loading test, the natural and 7D saturated specimen fragment sizes are large, and certain specimens retain the same shape as before testing. In fact, only small size differences exist between the broken particles. It should be noted that, as the specimen is fully confined during the triaxial tests,

TABLE 5: Quality percentage and fragment sizes of specimens.

Size	Quality													
	A1 (%)	A2 (%)	A3 (%)	A4 (%)	A5 (%)	A6 (%)	B1 (%)	B2 (%)	B3 (%)	B4 (%)	B5 (%)	B6 (%)	B7 (%)	B8 (%)
<30 mm	33	28.5	26	56	39	74	69	74.4	69.4	82.5	90.7	94.1	84.9	83.8
<10 mm	12	16	8	21	19	33	6.9	9.1	7.5	9.8	8.9	13.1	8.7	8.2
<5 mm	6.4	7.6	4.4	7.5	6.8	8	3.1	3.4	4.3	4.8	6.3	9.3	8.3	5.9

FIGURE 14: Failure forms of natural and 7D saturated specimens under uniaxial static loads.

FIGURE 15: Failure forms of natural and 7D saturated specimens under uniaxial static-dynamic loads.

FIGURE 16: Failure forms of natural and water-saturated specimens under triaxial static loads.

FIGURE 17: Failure forms of natural and water-saturated specimens under triaxial static-dynamic loads.

saturation has little influence on the broken particle sizes, as shown in Figures 16 and 17.

5. Conclusion

In this paper, the strength and failure characteristics of natural and water-saturated coal specimens are investigated. Static and static-dynamic loading tests, including uniaxial static and triaxial static loading, 1D static-dynamic loading, and 3D static-dynamic loading, are conducted using the RMT-150 and SHPB system. The main conclusions are as follows. (1) Under uniaxial compression and the 1D static-dynamic condition, the 7D saturated specimen strengths are lower than those of the natural specimens. (2) Under the triaxial static condition, the 7D saturated coal specimen strengths are lower than those of the natural specimens; however, in the 3D static-dynamic loading condition, the 7D saturated specimen strengths are higher than those of the natural specimens. (3) Under uniaxial compressional and 1D static-dynamic loading conditions, the broken particle sizes of the 7D saturated specimens are smaller than those of the natural specimens, indicating that saturation has a significant influence on particle breakage. However, under triaxial and 3D static-dynamic loading, saturation has no obvious influence on the broken particle sizes, due to the confinement effect.

Conflicts of Interest

The authors declare that there are no conflicts of interest regarding the publication of this paper.

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