

Research Article

The Impact of Traffic-Induced Bridge Vibration on Rapid Repairing High-Performance Concrete for Bridge Deck Pavement Repairs

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Based on forced vibration tests for high-performance concrete (HPC), the influence of bridge vibration induced by traveling vehicle on compressive strength and durability of HPC has been studied. It is concluded that 1 d and 2 d compressive strength of HPC decreased significantly, and the maximum reduction rate is 9.1%, while 28 d compressive strength of HPC had a slight lower with a 3% maximal drop under the action of two simple harmonic vibrations with 2 Hz, 3 mm amplitude, and 4 Hz, 3 mm amplitude. Moreover, the vibration had a slight effect on the compressive strength of HPC when the simple harmonic vibration had 4 Hz and 1 mm amplitude; it is indicated that the amplitude exerts a more prominent influence on the earlier compressive strength with the comparison of the frequency. In addition, the impact of simple harmonic vibration on durability of HPC can be ignored; this shows the self-healing function of concrete resulting from later hydration reaction. Thus, the research achievements mentioned above can contribute to learning the laws by which bridge vibration affects the properties of concrete and provide technical support for the design and construction of the bridge deck pavement maintenance.

1. Introduction

Due to rapid economic development in China, traffic volumes have significantly increased. Traffic loads on highways and bridges have continued to increase, which has caused significant damage to many bridge deck pavements and generated numerous heavy maintenance projects. During the process of highway bridge deck pavement renovation, bridge deck pavement repair projects should be performed without complete bridge closures (1/4 of the bridge is opened to traffic) to maintain efficient traffic flows. Because the repairing concrete endures constant impact from trafficinduced bridge vibration during the pouring, setting, and hardening processes, the impact of vibration on the mechanical performance and durability of repairing HPC for bridge deck pavement deserves our attention.

A concrete vibration test stand with a 48 Hz and an amplitude range of 0.5 mm–0.8 mm as the vibration source, one set of specimens was selected every 30 min from 90 min to 330 min after the concrete was poured to impose 30 s of vibration and to investigate the variation in the concrete's strength at 3 d, 7 d, and 28 d. The results showed that vibration prior to hardening can improve the compressive strength of concrete, whereas vibration after hardening can reduce the compressive strength of concrete [1]. Papers adopted a cement mortar vibration test stand as the vibration source. Under

the vibration conditions of 1 Hz and 4 Hz frequencies with 3 mm and 5 mm amplitudes, 30 min vibrations were imposed 1h, 2.5h, 3.5h, 4.5h, 6h, and 8h after pouring to measure the splitting tensile strengths of the specimens. The results showed that a vibration with a 5 mm amplitude imposed between initial hardening and final hardening reduced the tensile strength of the concrete [2, 3]. A rapping sieve is used as a vibration source and imposed a two-dimensional plane vibration condition with a lateral (3.68 Hz, 20 mm amplitude) and vertical (2.45 Hz, 4 mm amplitude) vibration. Vibrations that extended 10 min for every 60 min at intervals of 0-7 h, 7-14 h, and 14-24 h were imposed after pouring to determine the concrete's antiperturbation using an ultrasound method. The results showed that the concrete experienced an insignificant change in wave velocity under vibration conditions from 0-7 h and 14-24 h; however, the concrete strength under the vibration conditions decreased by an average of 2.2 MPa from 7 to 14 h [4–6]. Dunham et al. adopted a vibration test bed (VP5101, Syntron Corporation) as the vibration source. Under the condition of 60 Hz and peak vibration velocity ranges of 50-100 mm/s and 200-300 mm/s, 2 min vibrations were imposed at 2 h, 3.5 h, 4.5 h, 5 h, and 6 h after pouring. The splitting tensile strength and compressive strength of the specimen were measured for different concrete ages. The experimental results showed that vibration did not generate a significant impact on concrete strength [7]. In addition, papers conducted studies on the impact of vibration loading on concrete performance. Their findings concluded that vibration has an insignificant impact

on the late-stage strength of concrete [8–13]. These studies evaluated the impact of vibration on the concrete hardening process. However, several shortcomings in the previous studies have been identified: (1) trafficinduced bridge vibrations comprise a sustained, complex, and random process, because the vibration sources selected in the previous studies only provided singular vibration parameters and they cannot precisely simulate bridge vibration; (2) the impact from bridge vibration persists for a sustained interval during concrete hardening; in the previous studies, the vibration periods were too short to stimulate vibration; (3) during the process of highway bridge pavement renovation, the repairing concrete should achieve a high early-stage strength to ensure that the bridge can be opened to traffic as soon as possible, but the previous studies on the earlystage strength of concrete under the impact of vibration are limited; (4) in recent years, the durability of concrete has attracted considerable attention and the repairing concrete should possess satisfactory durability to improve its service life; however, few studies have focused on the impact of vibration on concrete durability. Therefore, to accurately analyze the impact of vibration on high-performance repair HPC for conditions in which the bridge is opened to traffic, this study used an electromagnetic vibration test bed with continuous vibration capability as artificial vibration source and configured different vibration parameters to simulate traffic-induced bridge vibration. The impact of vibration on the compressive strengths of different ages of rapid repairing high-performance concrete (HPC) (1d, 2d, 5d, 7d, and 28 d) and the 56 d durability of rapid repairing HPC were

also explored in different concrete hardening processes. The experimental results provided technical support for the repairing design and construction of bridge deck pavement without bridge closures and the importance of opening a bridge to traffic as soon as possible and ensuring superior quality of the repairing concrete.

2. Experimental Design

One of the key issues of the laboratory test for studying the influence of bridge vibration on the strength and durability of concrete is to choose reasonable simulation parameters to simulate bridge vibration. However, bridge vibration is a complex and random process that is impacted by various factors, such as vehicle loading size, speed, bridge surface flatness, and bridge structure. Therefore the accurate assessment of traffic-induced bridge vibration parameters is difficult and impractical. To simplify the vibration form, the simple harmonic vibration provided by an electromagnetic experiment test stand was adopted to simulate bridge vibration. Bridge vibration parameters were determined by combining the bridge vibration test results and the results of previous studies. Concrete hardening is a complex and gradually changing process. To study the impact of vibration on the performance of bridge deck pavement repairing HPC, this complex process should be divided into different phases based on the characteristics of concrete hardening, which can facilitate the analysis of the perturbation conditions for different concrete hardening phases using a targeted approach.

2.1. Determination of Vibration Parameters. To acquire reasonable simulation parameters from bridge vibration induced by traveling vehicle, the 891-4 type vibration pickup, QY-2 inclinometer, and G01NET type data acquisition instrument were adopted to perform vibration tests on the Jing-Zhang Highway K120+685 overpass. As shown in Figure 1, the bridge cross portfolio of the K120+685 overpass is 4×38 m; the upper section consists of a steel-concrete composite structure, and the lower section consists of a column pier. The vibration test entails measurements of the bridge's horizontal, lateral, and vertical vibration parameters under four conditions during which the bridge was open to traffic: prior to carving the bridge deck pavement, after carving the bridge deck pavement, after carving the bridge deck pavement and supporting the bottom section of the bridge, and after pouring the bridge deck pavement repairing HPC. In the vibration test, three 891-4 vibration sensors were installed at the middle of the main span to measure the vibration acceleration, velocity, displacement, and main vibration frequency of the bridge, as shown in Figure 1(a). Five QY-2 inclinometers were also installed on the main span to measure the dynamic deflection of the bridge, as shown in Figure 1(b). From the measurements, we derived the following results: for different bridge conditions during which the bridge is open to traffic, the vertical vibration is the maximum vibration, the frequency ranges from 2.5 to 5.5 Hz, and the vertical deflection ranges from 0.3 to 3 mm.





FIGURE 1: Bridge vibration measurement and test chart.

By investigating 224 different types of bridges, the Swiss Federal Research Laboratory determined that a bridge's actual measured natural frequency was f = 1.23-14 Hz with an average natural frequency of $f_m = 3.62$ Hz [14]. By combining the vibration data derived from the vibration test, the following representative vibration parameter combinations were established as experimental vibration parameters for this experiment: a frequency of 2 Hz with an amplitude of 3 mm, a frequency of 4 Hz with an amplitude of 1 mm, and a frequency of 4 Hz with an amplitude of 3 mm. An aircooled electromagnetic vibration test stand was used as the vibration output source, as shown in Figure 2. The maximum displacement peak value for the vibration test stand was 51 mm, and the excitation force ranged from 1 to 70 kN.

2.2. Division of Concrete Hardening Process. Based on cement concrete hardening theory [2], few hydration products are generated during the early phase of a cement-water reaction and the attractions among these products are relatively small. As time evolves, C–H–S forms long gel fibers, and a flocculation structure is created from the AFt and other hydration crystals between the cement particles. The cement paste begins to lose plasticity, which initiates the strength of concrete. As various hydrates significantly increase, the void spaces between the cement particles are filled with the hydration product to form an interlocking structure for which the strength continues to improve. As the cement paste loses plasticity, the hardening process begins, and cement forms at the beginning of the hardening phase.

During different hardening phases, concrete exhibits different vibration responses. Prior to the vibration experiment, highway engineering, cement, and concrete, test procedures were followed, and penetration resistance test meters were used to measure the change in the ultimate shear stress of the repairing concrete as a function of time [15].



FIGURE 2: Air-cooled electromagnetic vibration test stand.



FIGURE 3: Penetration resistance curve for the concrete sample.

The initial and final hardening times for the rapid repair highperformance concrete on the bridge deck pavement were determined. The curve for the penetration resistance test results is shown in Figure 3. The measurement results show that the initial and final hardening times for the repairing concrete are 7 h and 9 h. This experiment explored the conditions in which rapid repair high-performance concrete was perturbed during three phases: pouring-initial hardening (0– 7 h), initial hardening-final hardening (7–9 h), and pouringfinal hardening (0–9 h).

2.3. Experimental Groupings. Based on the previously mentioned analysis, three kinds of simple harmonic vibrations with different frequency and amplitude are imposed on the concrete specimens at pouring to the initial hardening phase, initial hardening to final hardening phase, and pouring to the final hardening phase, respectively. And then the compressive strength of the perturbed concrete at 1 d, 2 d, 5 d, 7 d, and 28 d, the electric flux at 56 d, and the freeze-thaw performance at 56 d were analyzed and compared to study the impact of vibration on the strength and durability of the HPC installed on the bridge deck pavement. The experimental groupings are described in Table 1.

Project	Control specimen/block	Vibration parameters	Pouring-initial hardening (0–7 h) specimen/block	Initial hardening-final hardening (7–9 h) specimen/block	Pouring-final hardening (0–9 h) specimen/block
		2 Hz, 3 mm	18	18	18
Compressive strength	18	4 Hz, 1 mm	18	18	18
		4 Hz, 3 mm	18	18	18
		2 Hz, 3 mm	3	3	3
Freeze-thaw	3	4 Hz, 1 mm	3	3	3
		4 Hz, 3 mm	3	3	3
Electric flux		2 Hz, 3 mm	3	3	3
	3	4 Hz, 1 mm	3	3	3
		4 Hz, 3 mm	3	3	3

TABLE 1: Experimental groupings.

2.4. Experimental Process. From a large number of laboratory experiments that are based on concrete workability and the performance requirement of rapid repair HPC, the following results can be derived regarding the benchmark ratio (mass ratio) of the HPC—cement: fly: slag: silicafume: sand: stone: water: acid water reducing agent: polypropylene fiber = 375:50:50:71:680:1115:155:6:1; water-paste ratio = 0.31; sand ratio = 0.38; initial degree of slumping = 200 mm and 1 h after initial slumping, the degree of slumping was 180 mm.

The experimental process and procedures to determine the impact of vibration on the strength of HPC are listed below.

- (1) Based on the experimental grouping, the HPC samples were installed into test molds with dimensions of $150 \text{ mm} \times 150 \text{ mm} \times 150 \text{ mm}$ after mixing.
- (2) The specimens for the pouring-initial hardening and pouring-final hardening phases were divide into two layers and installed on the experimental vibration test stand; each layer placed 18 concrete specimens. In addition, a positioning bolt was tightened to prevent the specimens from shaking, as shown in Figure 4. The initial hardening-final hardening specimens and the control group specimens were installed adjacent to the experimental test stand to maintain identical hardening conditions.
- (3) After one kind of vibration parameters was established (e.g., 2 Hz, 3 mm amplitude), the electromagnetic test stand was turned on. After forced vibration with 7 hours was applied to the concrete specimens, the vibration stand was paused, the experimental group for the pouring-initial hardening phase was removed, and the experimental group for the initial hardening-final hardening phase, which had been stored beside the vibration stand, was placed on the test stand. The test stand was turned on for 2 h of additional vibration and then turned off.
- (4) After completing one kind of vibration parameters experiments (e.g., 2 Hz, 3 mm amplitude), all experimental specimens were removed and placed into



FIGURE 4: Specimen fixation method.

a conservation room for cultivation. The concrete compressive strengths at different concrete ages (i.e., 1 d, 2 d, 5 d, 7 d, and 28 d) were measured.

(5) Utilizing this experimental procedure, experiments for measuring the impact of vibration on the mechanical performance of HPC were performed using the remaining two vibration parameter combinations.

For the experiments in which the impact on the durability of HPC was measured, premixed concrete (using the mentioned above experimental grouping methods) was placed into test molds with dimensions of 400 mm × 100 mm × 100 mm to measure the frost resistance of concrete and test molds with dimensions of φ 100 mm × 50 mm to measure the concrete flux of concrete. With the exception of the steps for measuring the 56 d frost resistance and electric flux, all experimental methods and steps for studying the impact of vibration on the durability and compressive strength of rapid repairing HPC were identical.

3. Experimental Results and Mechanism Analysis

3.1. Experimental Results for Mechanical Performance. A digital display press was used to measure the compressive strengths for various specimen groups; the data results are

Age/d	Control specimen/MPa	Pouring-initial hardening/MPa	Changes proportion/%	Initial hardening-final hardening/MPa	Changes proportion/%	Pouring-final hardening/MPa	Changes proportion/%
1	20.1	18.7	-7.1	18.3	-9.1	20.0	-0.6
2	35.2	32.9	-6.5	32.3	-8.2	34.0	-3.4
5	46.7	46.2	-1.1	46.2	-1.1	44.8	-4.1
7	49.3	49.4	+0.2	49.5	+0.4	49.4	+0.2
28	63.0	61.4	-2.5	61.1	-3.0	62.1	-1.4

TABLE 2: Compressive strength and rate of change of HPC specimens with vibration parameters of 2 Hz and 3 mm.

Note that "+" and "-" represent the decrease and increase of the compressive strength of the HPC specimens, respectively. And the same implication is in the subsequent tables.

TABLE 3: Compressive strength and rate of change of HPC specimens with vibration parameters of 4 Hz and 3 mm.

Age/d	Control specimen/MPa	Pouring-initial hardening/MPa	Changes proportion/%	Initial hardening-final hardening/MPa	Changes proportion/%	Pouring-final hardening/MPa	Changes proportion/%
1	20.1	18.7	-7.0	19.0	-5.6	19.5	-3.1
2	35.2	33.2	-5.7	33.2	-5.7	34.9	-0.8
5	46.7	46.3	-0.9	46.5	-0.4	45.7	-2.1
7	49.3	48.7	-1.2	47.3	-4.1	47.9	-2.8
28	63.0	64.1	+1.7	61.5	-2.4	62.0	-1.6

shown in Tables 2, 3, and 4. The corresponding timestrength change curves are shown in Figures 5, 6, and 7. The experimental results indicate that the compressive strengths for various vibration experiment groups at a HPC age of 28 d do not significantly differ from the compressive strengths for the still experiment group. To improve the display of the impact rule for early-stage concrete by vibration, Figures 5– 7 do not contain strength experiment data for the 28 d HPC age.

As shown in Table 2 and Figure 5, when the specimen experiences vibration with a 2 Hz and a 3 mm amplitude during the three phases of pouring-initial hardening, initial hardening-final hardening, and pouring-final hardening, the degree of impact by vibration exhibits similar trends. Compared with the compressive strengths for the still experiment group, compressive strengths of the specimens at 1 d and 2 d exhibited a slight reduction and the maximum reduction rate is 9.1%, the compressive strengths at 5 d, 7 d, and 28 d were slightly reduced, and the reduction amplitude also decreased as the age increased and the compressive strengths at 28 d decreased by 1.4%~3%. In addition, the average reduction rate of compressive strength of HPC specimens at 1d and 2 d at pouring-initial hardening phase, initial hardening-final hardening phase, and pouring-final hardening phase is 6.8%, 8.7%, and 2.0%, respectively.

For the specimen that experienced vibration with a 4 Hz and 3 mm amplitude, the experimental results showed trends similar to the trends for the specimen that experienced vibration with a 2 Hz and 3 mm amplitude, as shown in Table 3 and Figure 6. Compared with the compressive strengths for the still experiment group, the maximum reduction rate for compressive strength of HPC specimens at 1 d and 2 d was 7% and the maximum reduction rate for the 28 d compressive



FIGURE 5: Compressive strength and rate of change of HPC specimens with vibration parameters of 2 Hz and 3 mm.

strength was 2.4%. The average reduction rate of compressive strength of HPC specimens at 1 d and 2 d at pouring-initial hardening phase, initial hardening-final hardening phase, and pouring-final hardening phase is 6.4%, 5.7%, and 2.0%, respectively.

As shown in Table 4 and Figure 7, the vibration with the 4 Hz and 1 mm amplitude had a slight effect on the

Age/d	Control specimen/MPa	Pouring-initial hardening/MPa	Changes proportion/%	Initial hardening-final hardening/MPa	Changes proportion/%	Pouring-final hardening/MPa	Changes proportion/%
1	20.1	19.6	-2.5	20.2	+0.5	19.7	-2.1
2	35.2	34.2	-2.8	35.6	+1.1	36.0	+2.3
5	46.7	44.9	-3.9	46.7	+0.0	48.4	+3.6
7	49.3	50.3	+2.0	50.4	+2.2	48.7	-1.2
28	63.0	61.2	-0.29	62.4	-1.0	62.3	-1.1

TABLE 4: Compressive strength and rate of change of HPC specimens with vibration parameters of 4 Hz and 1 mm.



FIGURE 6: Compressive strength and rate of change of HPC specimens with vibration parameters of 4 Hz and 3 mm.

compressive strength of the HPC specimens. The 1 d and 2 d compressive strengths of the HPC specimens exhibited a change range of -2.8%~+2.3%. At 5 d, 7 d, and 28 d, the compressive strengths of the HPC specimens exhibited a change range of -3.9%~+3.6%.

The impact of vibration on concrete hardening is complex. Currently, no consensus has been formed in international studies about how vibration impacts concrete hardening. This study derived the following analyses based on the experimental results.

(1) The electromagnetic vibration test stand forced the concrete specimen to vibrate and enter an acceleration field that was represented by a sinusoidal curve. The coarse aggregate, fine aggregate, cementitious materials, and water in the concrete exhibited different responses to the acceleration. The vibration caused the coarse aggregate, cementitious materials, and water to separate. Due to the vibration of certain parameters, excessive free water emerged on the surface between the aggregate and the cementitious



FIGURE 7: Compressive strength and rate of change of HPC specimens with vibration parameters of 4 Hz and 1 mm.

materials, which slowed the hydration reaction in the concrete. After the vibration stopped, the concrete remained in this hardening condition. However, the above effect is not obvious at the condition of other vibrations. Therefore, compared with the still experiment group, the specimens in the vibration experimental group exhibited a reduced for concrete ages of 1 d and 2 d, in which the parameters of vibrations are 2 Hz, 3 mm amplitude, and 4 Hz, 3 mm amplitude. However, the effect is not obvious at the vibration with 4 Hz and 1 mm amplitude.

(2) Although vibration can weaken the interface between the aggregates and the cementitious materials, vibration does not cause distinct segregation of HPC aggregate, as demonstrated by the cross-sections of the HPC specimens (Figure 8); this finding may be attributed to the addition of polypropylene fibers in



(a) Still control group

(b) Vibration experimental group

FIGURE 8: Cross-section of specimen.

ГАВLE 5: Electric fl	ux and change	e ratio (<i>C</i>).
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Vibration parameters	Control specimen	Pouring-initial hardening	Changes proportion/%	Initial hardening-final hardening	Changes proportion/%	Pouring-final hardening	Changes proportion/%
F = 2 Hz $H = 3 mm$	333.0	349.9	+5.1	346.1	+3.9	340.5	+2.3
F = 4 Hz H = 3 mm	333.0	335.4	+0.7	345.3	+3.7	348.2	+4.6
F = 4 Hz $H = 1 mm$	333.0	328.8	-1.3	351.8	+5.6	337.8	+1.4

the HPC. Tens of millions of polypropylene fibers, which interconnect and bond to form an amorphous supporting system, were incorporated into the concrete. Polypropylene fibers simultaneously dissipated the concrete's shrink energy, which is associated with a high tensile strength and a low elastic modulus, onto the polypropylene fibers to prevent the expansion of the existing fragile components of the concrete to enhance the internal continuity of the material and to alleviate sinking of the aggregate sinking and the reduced strength caused by vibration. As the concrete age increased, the concrete continued to undergo a hydration reaction, and its fragile components were enhanced, which reflect the self-repairing performance of the concrete. Compared with the still experiment group, these analyses reasonably explain the phenomenon for which the compressive strengths of the specimen under the impact of vibration at 5 d, 7 d, and 28 d experienced minimal change and the amplitude gradually reduced as the age increased.

3.2. Experimental Results for Durability Performance. The experimental results obtained from the 56 d electric flux experiment and the 56 d 300-interval freeze-thaw cycle test are shown in Tables 5 and 6. The corresponding electric flux histogram is shown in Figure 9.



FIGURE 9: HPC electric flux and change ratio (*C*).

Table 5 and Figure 9 reveal that vibrations with different vibration parameters do not yield a distinct impact on the electric flux of HPC. Compared with the still experiment group, the vibration group yielded a maximum increase of 5.6% in the electric flux. As shown in Table 6, different vibrations with different parameters yield an insignificant impact on the condensation and antifreeze and thawing performance of concrete. For the 300-time freeze-thaw conditions, the

Vibration parameters	Control specimen	Pouring-initial hardening	Changes proportion/%	Initial hardening-final hardening	Changes proportion/%	Pouring-final hardening	Changes proportion/%
F = 2 Hz $H = 3 mm$	62.5	60.5	-3.2	63.1	+1.0	62.2	-0.5
F = 4 Hz H = 3 mm	62.5	61.4	-1.8	63.3	+1.3	62.2	-0.5
F = 4 Hz $H = 1 mm$	62.5	63.8	+2.1	61.8	-1.1	62.8	+0.5

TABLE 6: Freeze-thaw cycles with 300 dynamic modulus and change ratio.

measured dynamic elastic modulus values of various groups did not yield large differences; the dynamic elastic modulus values are in the range of $-3.2 \sim +2.1\%$. These results indicate that, due to the self-repair effect of the concrete's late-phase hydration reaction, the vibration had an insignificant impact on the durability of the repairing HPC; thus, the impact can be disregarded. The experimental results are consistent with the rules of impact of vibration on the late-phase compressive strength of HPC (e.g., 28 d compressive strength).

4. Conclusions

To examine the rapid repair of highway bridge deck pavement under the condition in which the bridge is open to traffic, this study adopted an electromagnetic test stand to simulate traffic-induced bridge vibration. By analyzing the impact on the compressive strength and durability of HPC, the following conclusions were obtained.

- (1) With the action of two simple harmonic vibrations with 2 Hz, 3 mm amplitude, and 4 Hz, 3 mm amplitude, the 1 d and 2 d compressive strengths of the HPC specimens decreased significantly, and the maximum reduction rate is 9.1%. As the age of the HPC increased, the HPC continued to undergo a hydration reaction and the fragile components inside the HPC expanded. Therefore, the impact of vibration on the late-phase strength of the HPC specimens was insignificant, which resulted in a loss of strength from 1.4% to 3%.
- (2) The simple harmonic vibration with 4 Hz and 1 mm amplitude has a slight effect on the compressive strength of HPC specimens. It is concluded that the amplitude of vibration has a prominent effect on the compressive strength of HPC specimens.
- (3) Due to the self-repairing effect of the hydration reaction during the late phase, the vibration had an insignificant impact on the durability of HPC, which can be disregarded. The experimental results are consistent with the vibration impact law for the late-phase compressive strength of HPC.
- (4) Based on the experimental results, if the traffic needs to be rapidly restored after bridge deck pavement repairs (e.g., 1 d or 2 d after pouring the HPC), the implementation of specific prevention measures, such

as heavy vehicle and speed restrictions and the installation of a bridge support, is recommended during the bridge deck pavement repair process to reduce the impact of traffic-induced bridge vibration on the early-phase strength of rapid repair HPC.

Under specific simplification conditions, this study experimentally investigated the impact of vibration on the mechanical properties and durability of HPC from a macro point of view. The impact of bridge vibration on the coagulation and hardening of bridge deck pavement repairing HPC is a complex process. To accurately describe and analyze the impact mechanism and rule, comprehensive macroand microexperimental studies on the forced vibration of concrete should be conducted in the future.

Conflict of Interests

The authors declare that there is no conflict of interests regarding the publication of this paper.

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