

Research Article

Shaped Charge Hydraulic Blasting: An Environmental, Safe, and Economical Method of Directional Roof Cutting

Quan Zhang¹, Jiong Wang², Shan Guo,² Weili Gong,² Longfei Feng,³ Haosen Wang,² Can Ming,² and Zimin Ma⁴

¹School of Mines, China University of Mining and Technology, Xuzhou, Jiangsu 221116, China ²State Key Laboratory of Geomechanics & Deep Underground Engineering, China University of Mining and Technology, Beijing 100083, China

³Center of Mine Disaster Control and Environment Management Technology, Xi'an Research Institute of China Coal Technology & Engineering Group Corp, Xi'an, Shanxi 710054, China

⁴School of Resources and Environmental Engineering, Shandong University of Technology, Zibo, Shandong 255049, China

Correspondence should be addressed to Quan Zhang; zhangquan@cumt.edu.cn and Jiong Wang; wangjiong0216@163.com

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Shaped charge blasting (SCB)—a directional fracture blasting technology—has the disadvantages of low safety, harmful gas, and dust emissions. This study proposes a new type of directional rock-breaking technology called shaped charge hydraulic blasting (SCHB) that uses water as the blasting medium instead of air. To verify the effectiveness of the new method, we performed a theoretical analysis. Twelve groups of on-site directional roof-cutting tests were carried out on SCB and SCHB for comparison in a coal mine. The results showed that, although both blasting methods achieved the goal of directional roof cutting for noncoal pillar mining, SCHB had greater blasting power and produced 8% higher crack rate than SCB for the same charge amount. SCHB required 12.5% less charge than SCB when the crack rate was basically the same. SCHB effectively reduced the concentration of CO. The maximum concentration of CO was reduced by 22–62% compared with SCB. The maximum concentration decreased parabolically with the monitoring distance. In addition, water can absorb high temperature and inhibit sparks from blasting, so SCHB can prevent gas explosions. Therefore, SCHB is an environmental, safe, and economical method of directional roof cutting, which has wide applications in mining and geotechnical engineering.

1. Introduction

Noncoal pillar mining does not need to manually or mechanically dig into the roadway. It automatically forms a roadway when coal is mined from the previous working face by roof cutting. It is known as the "the third mining science innovation" in China [1]. Directional roof cutting is one of the core technologies of this novel coal mining method. The principle of this method is shown in Figure 1.

Directional rock-breaking methods include soundless chemical demolition agents, water jets, directional hydraulic fracturing, slot hydraulic blasting, and shaped charge blasting (SCB). Soundless chemical demolition agents have been tested in the laboratory, and they can break the rock directionally [2, 3]. However, the demolition agents have the disadvantages of long reaction time [4], low power, and being easily affected by external temperature [5]. Therefore, it is not applied in directional roof cutting. The water jet can be precisely oriented to cut the roof, but it is difficult and time-consuming to fix the nozzle with this method. Directional hydraulic fracturing involves creating a groove in the borehole and then performing hydraulic fracturing [6–8]. To a certain extent, it can be oriented to precrack the roof, but the technology has a limited range of crack orientation extension. After the crack has spread along the prefabricated slot for a distance, the direction of crack propagation is controlled by the magnitude and direction of the *in situ* stresses. Slot hydraulic blasting uses a water jet to cut a slot of a certain



FIGURE 1: Schematic diagram of noncoal pillar mining with automatically formed roadway: (a) directional roof cutting and (b) roof collapses along the directional cutting line to form a roadway serving the next face.



FIGURE 2: Charge structure and directional roof-cutting mechanism of shaped charge hydraulic blasting (SCHB).

width and length in the rock and then performs ordinary blasting [9]. The stress change in the direction of the slot tip is greater than in other directions, which causes the crack to initiate and expand along this direction [10]. However, the scope of directional expansion is limited, and it takes time and effort to cut slots. Therefore, these methods cannot be used for directional roof cutting. At present, the directional roof-cutting technology applied to noncoal pillar mining with roadway formed automatically is mainly SCB [11–13]. However, this method emits toxic and harmful gases [14, 15], large dust [16, 17], and other explosive substances. The explosion itself produces CO [18–20]. CO, which has 250– 300 times the hemoglobin binding capacity of O_2 , is not filtered by the lungs and causes hypoxia in various tissues and



FIGURE 3: Test site (Dianping Coal Mine): (a) location in China and (b) 9-2042 lane.

cells of the human body. If the concentration of CO exceeds 24 ppm, it will present different degrees of harm in different people. When its concentration is too high, CO may cause asphyxiation or even death. To reduce the emission of harmful gases and dust, engineers developed the hydraulic blasting technology. Hydraulic blasting replaces the air medium around the explosive in the traditional blasting with water medium, using water to transfer the blasting energy [21–23]. Water can absorb harmful gases and dust [24, 25], but conventional hydraulic blasting produces multiple cracks in the rock mass [26–29], and the propagation direction and number of cracks cannot be controlled. Rock fracture [30–35] is one of the core problems of geotechnical engineering [36–41], but there are not many methods of directional rock fracture.

A new directional rock-breaking method called shaped charge hydraulic blasting (SCHB), which combines the advantages of SCB and hydraulic blasting, was proposed. The technology can generate cracks in a directional way, reduce the amount of toxic gas, and save the explosive charge. It is a green, safe, and economical method of directional rock breaking. The mechanism of SCHB directional roof cutting is analyzed. SCB and SCHB roof cutting were tested in a coal mine. The directional roof-cutting effect, crack rate, explosive dosage, and CO concentration of the two methods were compared.

2. Directional Roof-Cutting Mechanism of SCHB

The charge structure and directional roof-cutting mechanism of SCHB are shown in Figure 2. The detonation of the explosive material in water produces a high-speed water jet in the blast hole, which in turn impinges a directional impact in the direction of the shaped charge holes. This process is like the high-pressure water-jet rock breaking. Due to the incompressibility, higher density, and larger flow viscosity of water, the effect of the explosive shock waves in water is more intense and prolonged than in air. The explosion stress wave generated in the water decays more gradually and has a higher peak explosion pressure. Therefore, this method can save explosives. The directional slit tube changes the propagating direction of explosive blasting energy, which makes the wave, water jet, and explosive gas propagate focus on the direction of shaped charge holes. The directional slit tube produces concentrated tension in two directions, while producing uniform pressure in the rest. When the stress at the hole wall exceeds the dynamic tensile strength of the rock, the roof is split directionally along the holes. Compared with the SCB explosive, the SCHB directional slit tube and the hole wall use water as the coupling medium. Because of its high specific

Roof lithology	Uniaxial compressive strength/MPa	Uniaxial tensile strength/MPa	Cohesion/MPa	Internal friction angle/°
Mudstone	33.94	2.45	0.96	43.34
Fine sandstone	62.74	4.08	12.09	32.18

TABLE 1: Mechanical parameters of roof rock.



FIGURE 4: Roof and floor lithology and roof-cutting parameters of 9-2042 roadway.



FIGURE 5: Charge structure of shaped charge blasting (SCB): (a) connection and charge structure in a single hole, (b) an explosive roll, and (c) directional slit tube (1: detonator; 2: connecting wire; 3: explosive; 4: air; 5: directional slit tube; 6: sealing mud; 7: detonating equipment).



FIGURE 6: Charge structure of SCHB: (a) charge volume: 4.8 kg and (b) charge volume: 4.2 kg (1: detonator; 2: connecting wire; 3: explosive; 4: water; 5: directional slit tube; 6: sealing mud; 7: detonating equipment).

heat capacity, water is good at absorbing the heat of the explosive gas, has obvious flame-extinguishing effect, and can reduce the risk of gas explosion. In addition, water can effectively absorb toxic gases and blasting dust and thus improve the working environment.

In summary, SCHB rock breaking has three effects: a dynamic pressure effect and two static pressure effects. Dynamic pressure occurs before static pressure and is basically the shock waves in water [42]. One type of static pressure is the gas wedge effect of the explosive gas in the direction of the energy-accruing holes, and the other type of static pressure is the water wedge effect of the high-speed water jet. The effect of static pressure extends the cracks that have already started.

The pressure (P_S) of the shock wave in the water from the blasting point *R* is

$$P_{S} = 720\bar{R}^{-0.72},\tag{1}$$

$$\bar{R} = \frac{R}{\sqrt{Q_c}},\tag{2}$$

$$Q_c = \frac{\pi d_c^2 \rho_0}{4},\tag{3}$$

where *R* is the proportional distance, Q_c is the relative 2,4,6-trinitrotoluene equivalent of the explosive, d_c is the diameter of the explosive roll, and ρ_0 is the density of the explosive [43].

Combining Eqs. (1), (2), and (3), we get

$$P_{S} = \frac{720R}{\sqrt{\pi d_{c}^{2} \rho_{0}/4}}.$$
(4)

According to the detonation theory [44], the pressure (P_G) of the detonating gas in the blasthole is

$$P_G = P_K \left(\frac{P_W}{P_K}\right)^{4/9} \left(\frac{V_c}{V_b}\right)^{4/3},\tag{5}$$

$$P_W = \frac{1}{8}\rho_0 D^2,\tag{6}$$

$$\rho_0 V_c = \rho_b V_b, \tag{7}$$

$$\rho_{\rm b} = \frac{\rm q}{25\pi d^2},\tag{8}$$

where P_K is the critical pressure, P_W is the average detonation force, D is the detonation velocity of the explosive, V_b is the blasthole volume, V_c is the charge volume, ρ_b is the charge per unit volume of the blasthole, q is the charge concentration, and d is the borehole diameter.

Combining Eqs. (5), (6), (7), and (8),

$$P_G = P_K \left(\frac{\rho_0 D^2}{8P_K}\right)^{4/9} \left(\frac{q}{25\pi d^2 \rho_0}\right)^{4/3}.$$
 (9)



FIGURE 7: SCB tests: (a) single hole, (b) double holes, (c) four holes, and (d) six holes (1: detonator; 2: connecting wire; 3: explosive; 4: air; 5: directional slit tube; 6: sealing mud; 7: detonating equipment).



FIGURE 8: SCHB tests (single-hole charge 4.8 kg): (a) single hole, (b) double holes, (c) four holes, and (d) six holes (1: detonator; 2: connecting wire; 3: explosive; 4: water; 5: directional slit tube; 6: sealing mud; 7: detonating equipment).





FIGURE 9: SCHB tests (single-hole charge 4.2 kg): (a) single hole, (b) double holes, (c) four holes, and (d) six holes (1: detonator; 2: connecting wire; 3: explosive; 4: water; 5: directional slit tube; 6: sealing mud; 7: detonating equipment).

		8		
Blasting type	One hole charge/kg	Item Number of blastholes at one time	Number of CO monitoring sections	Distance of five CO monitoring sections from blasting zone/m
	4.8	1	5	100-200-300-400-500
Shaped charge blasting (SCP)	4.8	2	5	100-200-300-400-500
Shaped charge blasting (SCB)	4.8	4	5	100-200-300-400-500
	4.8	6	5	100-200-300-400-500
	4.8	1	5	100-200-300-400-500
Shaped charge hydraulic	4.8	2	5	100-200-300-400-500
blasting (SCHB)	4.8	4	5	100-200-300-400-500
	4.8	6	5	100-200-300-400-500
	4.2	1	0	None
CUD	4.2	2	0	None
SCHB	4.2	4	0	None
	4.2	6	0	None



FIGURE 10: Monitoring sections of CO concentration in lane 9-2042: 1-100 m from blasting zone, 2-200 m from blasting zone, 3-300 m from blasting zone, 4-400 m from blasting zone, and 5-500 m from blasting zone.



(a)

FIGURE 11: Effect of on-site directional roof cutting: (a) SCB and (b) SCHB.

The pressure (P_W) of the water jet [45] is

The water jet velocity is

$$P_W = \rho_w Q u, \tag{10}$$

where ρ_w is the density of water, Q is the flow rate of the water jet, and u is the velocity of the jet.

$$u = \frac{d_c^2 \rho_e}{d_t^2 \rho_w + d_c^2 (\rho_e - \rho_w)},\tag{11}$$

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TABLE 3: The crack rates of the 13 blastholes in SCB.

Hole number	1	2	3	4	5	6	7	8	9	10	11	12	13
Crack length on one side of the hole/m	5.81	5.95	6.09	5.67	5.81	5.88	6.30	5.61	5.95	5.74	6.16	5.60	6.09
Crack rate	83%	85%	87%	81%	83%	84%	90%	80%	85%	82%	88%	80%	87%

TABLE 4: The crack rates of the 13 blastholes in SCHB.

Hole number	1	2	3	4	5	6	7	8	9	10	11	12	13
Crack length on one side of the hole/m	6.51	6.23	6.72	6.16	6.09	6.65	6.79	6.37	6.44	5.95	6.44	6.72	6.65
Crack rate	93%	89%	96%	88%	87%	95%	97%	91%	92%	85%	92%	96%	95%

TABLE 5: Comparison of crack rate between SCB and SCHB.

Blasting category	Amount of charge in a hole	Average crack rate	verage Increased ck rate crack rate				
SCB	4.8 kg	84%	_				
SCHB	4.8 kg	92%	8%				

TABLE 6: Comparison of explosive dosage between SCB and SCHB.

Blasting category	Amount of charge in a hole	Average crack rate	Save explosives
SCB	4.8 kg	84%	_
SCHB	4.2 kg	85%	12.5%

where d_t is the outer diameter of the directional slit tube and ρ_e is the density of the detonation product.

Combining Eqs. (10) and (11),

$$P_W = \rho_w Q \frac{d_c^2 \rho_e}{d_t^2 \rho_w + d_c^2 (\rho_e - \rho_w)}.$$
 (12)

In summary, the stress *P* at the crack is

$$P = P_{S} + P_{G} + P_{W} = \frac{720R}{\sqrt{\pi d_{c}^{2} \rho_{0}/4}^{-0.72}} + P_{K} \left(\frac{\rho_{0} D^{2}}{8P_{K}}\right)^{4/9} \left(\frac{q}{25\pi d^{2} \rho_{0}}\right)^{4/3} + \rho_{w} Q \frac{d_{c}^{2} \rho_{e}}{d_{t}^{2} \rho_{w} + d_{c}^{2} (\rho_{e} - \rho_{w})}.$$
(13)

3. SCHB Directional Roof-Cutting Tests

3.1. Test Site. Dianping Coal Mine is located in Dianping Village, Dawu Town, Fangshan County, Luliang City, Shanxi Province, China (Figure 3). It is a low-gas mine. The coal dust in each coal seam has the danger of explosion, and all coal seams are spontaneously ignited. The test site was the 9-2042 return air lane of the 9-204 coal mining face. The 9-2041 roadway was formed automatically by roof cutting in the 9-202 working face. The length of the 9-2042 reserved lane was 1,644 m, and the test was from 500 m to 1000 m away from the open-off cut. The cross section of 9-2042 roadway was rectangular, 4.6 m long, and 3.1 m high. 9-204 working face adopted W-type ventilation. The wind speed was 1.8 m/s, and the air volume flow rate was $350 \text{ m}^3/\text{min}$. The return air shaft was 530 m away from the test site.

3.2. Test Scheme. The SCB and SCHB tests were performed in the 9-2042 lanes. The immediate roof above the coal seam was mudstone, and the main roof was fine sandstone. The mechanical parameters of the roof rock are shown in Table 1. The roof-cutting depth was 9m, and the angle between the blastholes and the vertical direction was 17° (Figure 4). The charge structure of SCB is shown in Figure 5. The length of the sealed hole was 2 m, and the total length of the directional slit tube was 7 m. There were a total of five directional slit tubes with lengths of 1.5 m, 1.5 m, 1.5 m, 1.5 m, and 1 m. The five tubes were loaded with charges that weight 1.2 kg, 1.2 kg, 0.9 kg, 0.9 kg, and 0.6 kg, and a single-hole charge weighed 4.8 kg. An explosive roll was 300 mm long, 35 mm in diameter, and weighed 0.3 kg. The directional slit tube had an inner diameter of 36.5 mm, an outer diameter of 42 mm, and a hole diameter of 48 mm.

As shown in Figure 6, there are two types of SCHB charge structure. The first structure has a single-hole charge of 4.8 kg and five directional slit tubes with charges of 1.2 kg, 1.2 kg, 0.9 kg, 0.9 kg, and 0.6 kg. The other has a single-hole charge of 4.2 kg with charge amounts of 1.2 kg, 0.9 kg, 0.9 kg, 0.6 kg, and 0.6 kg. Compared with the charge structure of SCB, SCHB uses water bags in the directional slit tubes. The rest of the conditions were the same as SCB's. Both types of SCHB used 1, 4, 3, and 3 water bags in four directional slit tubes, respectively. The last tube was not put into the water bag, so as not to damage the water bag when loading the gun mud. In single-hole SCB (Figure 7(a)), double-hole SCB (Figure 7(b)), four-hole SCB (Figure 7(c)), and sixhole SCB (Figure 7(d)), the charge structures of all holes are the same. In single-hole SCHB (Figures 8(a) and 9(a)), double-hole SCHB (Figures 8(b) and 9(b)), four-hole SCHB (Figures 8(c) and 9(c)), and six-hole SCHB (Figures 8(d) and 9(d)), the charge structures of all holes are the same.

SCHB tests with a hole charge of 4.8 kg were conducted for comparing the results with the directional roof-cutting effect, crack rate, and CO gas concentration of the SCB tests. SCHB tests with a hole charge of 4.2 kg were used for comparing the results to those with the SCB explosive charge.



FIGURE 12: Time-varying curve of CO concentration in single-hole blasting at different measuring points: (a) 100 m from blasting zone, (b) 200 m from blasting zone, (c) 300 m from blasting zone, (d) 400 m from blasting zone, and (f) 500 m from blasting zone.

In the single-hole, double-hole, four-hole, six-hole SCB, and SCHB (one-hole charge was 4.8 kg) test processes, five roadway sections monitored CO concentration (Table 2). Five CO concentration monitoring sections were at distances of 100 m, 200 m, 300 m, 400 m, and 500 m from the blasting zone (Figure 10). A test point was arranged on each section.



FIGURE 13: Changes in maximum CO concentration with monitoring distance in single-hole SCB and SCHB.



FIGURE 14: Time taken by CO to reach 24 ppm and exceed the same along the monitoring distance in single-hole SCB and SCHB.

Before and after the tests, the borehole peeper was used to observe cracks in the hole, and the crack rates of the two blasting methods were compared and analyzed.

3.3. Test Process. A row of holes was drilled in the roof, and then, the explosives were loaded into the directional slit tubes. Water bags were placed in the directional slit tubes during the SCHB test. The tubes were then put into the boreholes, and the shape charge holes of the directional slit tubes were oriented along the direction of directional presplitting. After the five directional slit tubes filled with explosives were filled in the drill holes, the holes were sealed with gun mud. When sealing the holes, the gun mud was tamped to prevent the explosives from breaking away and influencing the blasting effect. After the charge loading was completed, the blastholes were detonated. At the time of detonation, the CO concentration detectors at distances of 100 m, 200 m, 300 m, 400 m, and 500 m from the blasting site needed to be turned on simultaneously to record the CO concentration data. In addition, the dust concentration was monitored at a distance of 300 m from the blasting area. Furthermore, a peeper was used to observe the cracks in the holes before and after the tests.

4. Test Results and Discussion

4.1. Directional Roof-Cutting Effect. Before the blast, there were no cracks in the hole. The cracks generated by SCB and SCHB after the blast are shown in Figures 11(a) and 11(b), respectively. Neither blasting methods could break the hole wall. There were, however, two cracks in it. Both could achieve the effect of directional roof cutting. This was because, whether it was SCB or SCHB, the orientation effect was achieved by the directional slit tube. The directional slit tube changed the balance of impact of the shock wave on the hole wall in the initial stage. Therefore, energy from the shock wave was mainly concentrated in the direction of the two rows of shape charge holes, and tangential tensile stress was formed on the hole wall. The initial cracks were formed when the tensile stress at the hole wall exceeded the dynamic tensile strength of the rock. Subsequently, when the stress intensity factor at the crack tip was greater than the dynamic fracture toughness of the rock, the crack expanded in a predetermined direction.

SCB relied on the air medium to couple. SCHB's coupling medium was mainly water and a small amount of air. The SCB rock-breaking mechanism used shock wave and gas pressure, while the SCHB rock-breaking mechanism has three modes—shock wave, gas pressure, and water pressure. Although SCHB had more water wedges than SCB, and it was less destructive, these waves and forces were still mainly concentrated in the direction of the shape charge holes, and the rock mass in the other directions was protected by the energy-gathering pipes. Therefore, the crack-propagation direction of the rock mass in the two blasting modes was the same, and the directional effect was independent of the coupling medium.

4.2. Crack Rate. The crack ratio is the ratio of the length of the crack generated on one side of the borehole to the length of the directional slit tubes. The self-forming roadway without coal pillars required that the cutting rate be no less than 80%. The crack rates of the 13 blastholes in SCB were 83%, 85%, 87%, 81%, 83%, 84%, 90%, 80%, 85%, 82%, 88%, 80%, and 87% (Table 3). The average crack rate was 84%. The crack rates in thirteen boreholes in SCHB were 93%, 89%, 96%, 88%, 87%, 95%, 97%, 91%, 92%, 85%, 92%, 96%, and 95% (Table 4). The average crack rate was 92%. Although the two blasting methods have the same charge, the SCHB replaced part of the air medium with water, and the crack rate was increased by 8%.

The increase in crack rate was conducive to the timely collapse of the roof along the cutting line, optimized the stress environment, reduced the support resistance of the roadway behind the working face, and weakened the roadway



FIGURE 15: Time-varying curve of CO concentration in double-hole blasting at different measuring points: (a) 100 m from blasting zone, (b) 200 m from blasting zone, (c) 300 m from blasting zone, (d) 400 m from blasting zone, and (f) 500 m from blasting zone.

deformation. The power of explosives in water was much greater than that in air. On the one hand, the intensity of shock waves in water was greater. The initial pressure of the shock wave in the air ranged from 0.8 to 1.3 MPa, while reaching 100 MPa in water [46]. Thus, the intensity of the shock wave in water increased by nearly a hundred times.

On the other hand, the static water instantly became a highspeed water jet after the explosive was detonated, and its impact force on the rock was large. However, the SCB did not exhibit the water wedge effect. Therefore, SCHB had a higher crack rate than SCB, and their comparison is drawn in Table 5.

4.3. Explosive Dosage. Under the condition that the crack rate was no less than 80%, the total charge in a borehole for SCHB was changed to 4.2 kg, to reduce the costs. According to the charge loaded, the crack rates of the 13 blastholes were 85%, 86%, 88%, 83%, 87%, 80%, 81%, 91%, 86%, 89%, 82%, 85%, and 87%, and the average crack rate was 85%. This is similar to the crack rate of traditional SCB with a single-hole charge of 4.8 kg. Therefore, to achieve the same roof-cutting effect, the charge of SCHB weighed less than that of traditional SCB, which could save 12.5% of the explosive charge (Table 6).

There are two ways in which SCHB saves explosive mass. (1) Due to the low compressibility and high density of water, the peak pressure of the shock wave generated by the detonation of the same amount of explosive in water was much higher than in air. SCHB could produce a large impact force on the hole wall. In addition, the expansion rate of the water explosion products was slower than that in air, so the quasistatic stress field formed by SCHB took a long time to act [47]. Therefore, water coupling increases the stress at the crack tip and prolongs the time of effect of crack propagation. As a result, the length of crack propagation and the blasthole spacing increased. (2) SCHB increased the rockbreaking effect of high-speed water jets. After the explosion, the water in the water bag was ejected at an extremely high velocity along the energy gathering holes. When the highspeed water jet speed reached 500 m/s, the impact pressure was about 300 MPa. Moreover, 14 mg of explosives exploded in water, resulting in a jet velocity of 700 m/s [48]. The charge of this test was 4.2 kg, so its impact force was greater.

4.4. Economic Cost. The length of this roadway was 1,644 mm. The SCB had a blasthole spacing of 400 mm, a number of blastholes of 4,110, and required 19,828 kg of explosives. The distance between adjacent blastholes for SCHB was 500 mm, the number of blastholes was 3288, and 13810 kg of explosives was required. Therefore, the entire tunnel saved 5918 kg of explosives. If the explosive price was 20 RMB/kg, it could save 118,360 RMB. The number of drill holes for SCHB was 822 less than that of SCB, which saved about 130,000 RMB in labor cost and improved construction efficiency. The total cost of the water bag for the SCHB was about 3500 RMB. Therefore, the SCHB technology has saved this roadway 127,860 RMB. In summary, the SCHB could greatly save the amount of explosives, reduce the economic cost, and improve the operation efficiency.

4.5. CO Concentration

4.5.1. Single-Hole Blasting. The CO concentration of SCB and SCHB at 100 m, 200 m, 300 m, 400 m, and 500 m from the blasting site changed with time, as shown in Figure 12. All the five monitoring points indicated that the CO concentra-



FIGURE 16: Changes in maximum CO concentration with monitoring distance in double-hole SCB and SCHB.



FIGURE 17: Time taken by CO to reach 24 ppm and exceed the same with the monitoring distance in double-hole SCB and SCHB.

tion was divided into the normal stage, the rapid increase stage, the sharp decline stage, the slow decline stage, and returning-to-the-normal stage. After the blasting, it took a while for CO to reach the monitoring location. Before CO reached the monitoring point, its concentration at the monitoring point was the same as before the blasting (normal level). This period was called the normal stage. After reaching the monitoring point, its concentration rose rapidly to the highest possible value. This stage was called the rapid increase stage. Beyond this value, the concentration of CO decreased rapidly, which was called the sharp decline stage. When the concentration of CO dropped to a certain value, its rate of decline slowed down. This was called the slow decline stage. As fresh air continued to dilute the cannon smoke, the CO concentration returned to the same size as from before the blast. This was called the return-to-normal stage.



FIGURE 18: Time-varying curve of CO concentration in four-hole blasting at different measuring points: (a) 100 m from blasting zone, (b) 200 m from blasting zone, (c) 300 m from blasting zone, (d) 400 m from blasting zone, and (f) 500 m from blasting zone.

The maximum concentration of CO in SCB and SCHB showed a parabolic decline with increasing monitoring distance (Figure 13). The CO inhibition rate was introduced to

analyze the reduction of CO concentration after the SCHB. The maximum concentration of CO at a certain monitoring point for SCB was M_1 . The maximum concentration of CO

at the same monitoring point for SCHB was M_2 . Therefore, the CO inhibition rate is

$$\mu = \frac{M_1 - M_2}{M_1} \times 100\%. \tag{14}$$

The CO inhibition rates at 100 m, 200 m, 300 m, 400 m, and 500 m were 47%, 39%, 32%, 22%, and 22%, respectively. Therefore, the SCHB could significantly reduce the CO concentration, optimize the working environment, and benefit the health of workers. The threshold limit of CO concentration in China is 24 ppm. At different monitoring sites, the SCHB reached 24 ppm after the SCB (Figure 14). Within 100-500 m of the blasting area, the time taken by the CO concentration to exceed threshold of SCHB was shorter than that in SCB. Moreover, the time taken by the SCB CO concentration to exceed the threshold did not exceed 3.13 min, while the time taken by the SCHB CO concentration to exceed the same did not exceed 2.25 min. Due to the small charge of single-hole blasting, the CO concentration took less time to exceed the threshold. Therefore, CO entered the blasting area earlier.

4.5.2. Double-Hole Blasting. After the double-hole initiation, the CO concentration at a monitoring point also went through five stages with time: the normal stage, the rapid increase stage, the sharp decline stage, the slow decline stage, and return-to-normal stage (Figure 15). The total time of hese five stages did not exceed 16 min. Compared with single-hole blasting, the charge amount of doublehole blasting was doubled, so the amount of CO generated was also higher than that of single-hole blasting at the five monitoring points. The peak value of the CO concentration decreased with the increase of the monitoring distance, and the peak value decreased rapidly thereafter. At the same moment, in any given location, the CO concentration produced by the SCHB was not greater than that of the SCB. This shows that the water medium coupling is better than the air medium, which could reduce the percentage of toxic and harmful gases.

The relationship between the maximum CO concentration and the monitoring distance for SCB and SCHB (Figure 16) is

$$y = 5.57E - 4x^2 - 0.58x + 323.4,$$
 (15)

$$y = 1.86E - 4x^2 - 0.23x + 134.8.$$
(16)

At distances of 100 m, 200 m, 300 m, 400 m, and 500 m from the blasting zone, the maximum CO concentrations of SCB were 271 ppm, 230 ppm, 198 ppm, 182 ppm, and 172 ppm, while the maximum CO concentrations of SCHB were 113 ppm, 96 ppm, 82 ppm, 70 ppm, and 65 ppm, respectively. Therefore, the SCHB CO inhibition rates at these five monitoring points were 58%, 58%, 59%, 62%, and 62%, respectively. As shown in Figure 17, the CO concentration overrun time of SCB was from 1.87 to 4.13 min, and that of SCHB was between 1.23 and 3.35 min. At the five monitoring points, the CO concentration overrun time period of SCHB



FIGURE 19: Changes in maximum CO concentration with monitoring distance in four-hole SCB and SCHB.



FIGURE 20: Time taken by CO to reach 24 ppm and exceed the same with monitoring distance in four-hole SCB and SCHB.

was shorter than that of SCB. In addition, CO of SCHB reached the over-limit concentration slightly slower than that of SCB. This was because SCHB produced less CO. When the air volume and wind speed were constant, the time required for the CO gas mass to move from the blast source to each monitoring point was proportional to the distance. Therefore, the moment at which the over-limit concentration was reached was approximately linear with the monitoring distance.

4.5.3. Four-Hole Blasting. The charge of four-hole blasting was four times that of single-hole blasting and twice that of double-hole blasting. Therefore, the CO concentration from the four-hole blasting was also much greater than that of the single- and double-hole blasting methods (Figure 18). When the air volume was constant, the higher the amount



FIGURE 21: Time-varying curve of CO concentration in six-hole blasting at different measuring points: (a) 100 m from blasting zone, (b) 200 m from blasting zone, (c) 300 m from blasting zone, (d) 400 m from blasting zone, and (f) 500 m from blasting zone.

of explosives detonated simultaneously, the longer the ventilation time required to dilute the CO in the gun smoke. During blasting, the atomization of water caused the water and CO to come in full contact. Under the high temperature and high pressure generated by the blasting, water and CO generated $\rm CO_2$ and $\rm H_2$, thereby greatly absorbing CO, that is,

$$CO + H_2O = CO_2 + H_2$$
(high temperature). (17)



FIGURE 22: Changes in maximum CO concentration with monitoring distance in six-hole SCB and SCHB.



FIGURE 23: Time taken by CO to reach 24 ppm and exceed the same with the monitoring distance in six-hole SCB and SCHB.

The relationship between the maximum CO concentrations of SCB and SCHB with the monitoring distance (Figure 19) is

$$y = 9.57E - 4x^2 - 1.17x + 693, \tag{18}$$

$$y = 3.21E - 4x^2 - 0.44x + 281.$$
(19)

The coefficients of determination of these two curves were 0.9973 and 0.9982, which are close to 1. This shows that the relationship between the maximum CO concentration and the monitoring distance is a parabola, and the goodness of fit is high. The CO inhibition rates at the five monitoring points were 59%, 59%, 58%, 59%, and 59%, respectively. The water medium significantly inhibited CO and reduced its movement in the lane. The time for CO to reach the over-limit concentration was almost the same in the two blasting methods (Figure 20). At the five monitoring points, SCB's overrun durations were 3.77, 3.28, 3.88, 5.58, and 5.78, while those of SCHB were 2.3, 2.23, 3.23, 2.88, and 3.7, respectively. Both blasting methods had the same amount of ventilation, but SCB produced more CO. Hence, SCB's CO overrun time was longer than SCHB.

4.5.4. Six-Hole Blasting. The CO concentration law of six-hole blasting was basically the same as those of the single-hole, double-hole, and four-hole blasting methods (Figures 21-23). However, it was just that the amount of CO produced by SCB and SCHB increased as the charge increased. The amount of CO produced by blasting was proportional to the quantity of explosives. Therefore, when the amount of ventilation was constant, the quantity of explosives should be strictly controlled. This can effectively reduce the amount of toxic gas CO, thereby shortening the time of CO migration and diffusion. The CO suppression rates at the five monitoring points were 50%, 49%, 45%, 42%, and 46%, respectively. The short-term overrun of CO was inevitable, but the water mist formed by SCHB at the time of explosion reacted with CO to generate CO_2 and H_2 , which could effectively decrease the CO concentration.

5. Conclusion

SCHB, as a novel directional roof-cutting technology, uses water as the blasting medium instead of air. It is an environmental, safe, and economical method of directional roof cutting. Conclusions are as follows:

- Compared with SCB, SCHB reduces the quantity of explosives required, increases the crack rate of the rock, and saves costs. Therefore, it is a more economical blasting technology
- (2) SCHB can decrease the concentration of toxic gases CO, which is beneficial to the health of workers. Thus, it is a green blasting method
- (3) Water can absorb the heat generated by blasting. SCHB can reduce the possibility of gas explosion. Hence, it is a safe method

Data Availability

The data used to support the study can be obtained from the authors.

Conflicts of Interest

The authors declare that there is no conflict of interest regarding the publication of this paper.

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