Optimal design and control of mechanical systems with uncertain input

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Abstract. This paper presents an integrated approach to optimize for design and control of mechanical systems with random input parameters. Random parameters are represented by probability density functions. A numerical technique defining directly the representative values and the associated probabilities is implemented by modeling the stochastic parameters as generalized Wiener processes. The Monte Carlo's method is also implemented to deal with correlated parameters. A design and control sensitivity analysis optimization formulation is derived. A conceptual separation between time variant and time invariant design parameters is presented, this way including the design space into the control space and considering the design variables as control variables not depending on time. By using time integrals through all the derivations, the design and control problems are unified. In the optimization process we can use both types of variables simultaneously or by interdependent levels. The dynamic response is modeled via space and time finite elements, and is integrated either by at-once integration or step-by-step. The formulation is applied to a numerical example.

Keywords: Space-time finite elements, adjoint system, stochastic modeling, wiener processes

1. Stochastic modeling

Most of the optimum design is applied to systems under deterministic loading. However, much of the decision making in real world deals with systems that may not be precisely described. When the system parameters are described as random variables through statistics as mean and standard deviation, a probabilistic approach can be used for the analysis and design of the system [8]. This paper presents an approach to optimize for designing of mechanical systems with random input parameters. The random parameters are characterized by specifying their probability density functions.

Due to their multivariate nature, integrations involving these functions are in general impracticable. However, this limitation may be overcome by adopting a methodology that makes possible to substitute the continuous distributions by discrete distributions with identical expected values and covariance matrices. In this methodology, the random parameters are described by generalized Wiener processes.

At the present state of the research work, we consider that only the dimensions of the obstacles encountered by the system are random. The location of the obstacles is assumed deterministic.

Under these conditions, the value g of the obstacles is obtained from one of the following stochastic differential equations:

$$dg(\tau, W) = \mu d\tau + \sigma dW \quad (1)$$

$$dg(\tau, W) = \mu g d\tau + \sigma g dW \quad (2)$$

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where μ and σ^2 are constants, W represents a Brownian motion and τ is the time. The time here is an auxiliary variable that represents the point from which the dimensions of a given obstacle have been grown continuously up to the present value.

The first process leads to a Gaussian distribution while the second one leads to a Log-Normal distribution. We assumed that the obstacles have only positive dimensions and adopted the second equation, which has the following solution:

$$g_\tau = g_0 \exp\left[\left(\mu - \sigma^2/2\right)\tau + \sigma W_\tau\right] \quad (3)$$

The expected value and the variance of g are then given by:

$$E[g_\tau] = g_0 \exp(\mu\tau) \quad (4)$$

$$\text{Var}[g_\tau] = E[g_\tau]^2 [\exp(\sigma^2\tau) - 1] \quad (5)$$

From these equations, the parameters μ and σ^2 can be obtained by specifying the expected value and the variance of g .

To obtain the discrete distribution, the temporal evolution of the logarithm of g was simulated using trinomial movements [9]. The use of this transformation is justified due to its advantage of leading to constant elemental probabilities of evolution. The corresponding process is obtained using the Ito's lemma [10] and it is represented by the following equation:

$$df = \mu'' d\tau + \sigma dW \quad (6)$$

where $\mu'' = \mu - \sigma^2/2$ and $f = \ln(g)$. Since the first and second moments of the continuous and discrete distributions coincide, the following equations for the probabilities are obtained as [3,4]:

$$P_d = \frac{\sigma^2 \Delta t + \eta^2 - \eta \Delta f_u}{\Delta f_d (\Delta f_d + \Delta f_u)} \quad (7)$$

$$P_m = 1 - \frac{\sigma^2 \Delta t + \eta^2 - \eta (\Delta f_u - \Delta f_d)}{\Delta f_d \Delta f_u} \quad (8)$$

$$P_u = \frac{\sigma^2 \Delta t + \eta^2 + \eta \Delta f_d}{\Delta f_u (\Delta f_d + \Delta f_u)} \quad (9)$$

where the parameters involved are represented graphically in the Fig. 1.

Fig. 1 Trinomial evolution of the stochastic variable

where $\eta = E[\Delta f] - (f_m - f_t)$, $\Delta f_d = f_m - f_d$ and $\Delta f_u = f_u - f_m$. The values of Δf can be chosen using the condition of identical probabilities and the value of Δt determines the number of points obtained. Using a uniform

grid and several time steps, the probabilities of the ending nodes can be obtained applying the following recurrence relation:

$$P_i^t = P_u P_{i-1}^{t-1} + P_m P_i^{t-1} + P_d P_{i+1}^{t-1} \quad (10)$$

Finally, after the definition of this distribution, the value of an obstacle dimension at a given location has been obtained by selecting randomly one of those values using the accumulated probabilities (i.e. the distribution function) and a corresponding random value obtained from a uniform distribution between 0 and 1.

2. Analysis and design sensitivity analysis modeling

For the dynamic response analysis and design sensitivity analysis, space and time finite element discretizations are included in the same system of equations [11] such that space and time integrations can be performed simultaneously by at-once integration as for static cases. Design sensitivity analysis is implemented throughout the adjoint system approach [5].

2.1. Analysis

A space-time finite element model is used to discretize the dynamic analysis response. After space discretization we have the governing matrix equation as

$$\mathbf{M}^t \ddot{\mathbf{U}} + {}^t\mathbf{C}_S \dot{\mathbf{U}} + {}^t\mathbf{K}_S \mathbf{U} = {}^t\mathbf{P} \quad (11)$$

where $\mathbf{M}$ is the mass matrix, ${}^t\mathbf{C}_S \equiv {}^t\mathbf{C}_S({}^t\dot{\mathbf{U}})$, ${}^t\mathbf{K}_S \equiv {}^t\mathbf{K}_S({}^t\mathbf{U})$ are respectively the damping and stiffness matrices, ${}^t\mathbf{P}$ is the loading vector and ${}^t\mathbf{U}$, ${}^t\dot{\mathbf{U}}$, ${}^t\ddot{\mathbf{U}}$ are respectively the displacement, velocity and acceleration vectors, all the quantities defined at time t .

For temporal modeling, finite elements of dimension Δt were considered, selecting hermitean cubic elements to model the displacements and quadratic lagrangean elements to model the loading, extending the algorithm given in [7] to the case of nonlinear systems. By taking the time derivative of the Eq. (11) by one hand, and by another hand its integration once and then twice with average values of stiffness and damping given as

$$\bar{\mathbf{K}} \equiv {}^{t+\Delta t/2}\mathbf{K}_S({}^t\mathbf{U} + {}^t\dot{\mathbf{U}} \Delta t/2), \quad \bar{\mathbf{C}} \equiv {}^{t+\Delta t/2}\mathbf{C}_S({}^t\dot{\mathbf{U}} + {}^t\ddot{\mathbf{U}} \Delta t/2) \quad (12)$$

one obtains four equations that combine to give the dynamic finite time-element equation as

$$\mathbf{D}_S^e \mathbf{z}^e = \mathbf{P}^e \quad (13)$$

where

$$\mathbf{D}_S^e = \begin{bmatrix} {}^t\mathbf{K}_S & {}^t\mathbf{C}_S & \mathbf{M} & \mathbf{0}_{n \times n} & \mathbf{0}_{n \times n} & \mathbf{0}_{n \times n} \\ \hline {}^t\mathbf{D}_{S_{11}} & {}^t\mathbf{D}_{S_{12}} & \mathbf{0}_{n \times n} & {}^{t+\Delta t}\mathbf{D}_{S_{11}} & {}^{t+\Delta t}\mathbf{D}_{S_{12}} & \mathbf{0}_{n \times n} \\ \hline {}^t\mathbf{D}_{S_{21}} & {}^t\mathbf{D}_{S_{22}} & \mathbf{0}_{n \times n} & {}^{t+\Delta t}\mathbf{D}_{S_{21}} & {}^{t+\Delta t}\mathbf{D}_{S_{22}} & \mathbf{0}_{n \times n} \end{bmatrix}, \quad \mathbf{P}^e = \begin{Bmatrix} {}^t\mathbf{P} \\ \mathbf{P}_1 \\ \mathbf{P}_2 \end{Bmatrix} \quad (14)$$

and

$$\mathbf{z}^e = ({}^t\mathbf{z}, {}^{t+\Delta t}\mathbf{z}), \quad {}^t\mathbf{z} = ({}^t\mathbf{U}, {}^t\dot{\mathbf{U}}, {}^t\ddot{\mathbf{U}}) \quad (15)$$

In Eq. (14) n stands for number of space degrees-of-freedom and ${}^t\mathbf{D}_{S_{jk}}$ are functions of $\bar{\mathbf{K}}, \bar{\mathbf{C}}, \mathbf{M}$.

The Eq. (13) may be solved step-by-step, i.e., element-by-element in time, or assembled to be solved at-once. In this case, we have to assemble for a total time interval T discretized in N time nodes, resulting in the dynamic equation

$$\mathbf{D}_S \mathbf{z} = \mathbf{P} \quad (16)$$

where $2n$ time boundary conditions are imposed by transferring the corresponding columns of the assembled matrix $\mathbf{D}_S$ to the right-hand side of Eq. (16) after multiplying by the vector $\mathbf{U}_c$ of those conditions, resulting the equation

$$\hat{\mathbf{K}}_S \hat{\mathbf{U}} = \hat{\mathbf{P}}, \quad \hat{\mathbf{P}} = \mathbf{P} - \mathbf{D}_c \mathbf{U}_c \quad (17)$$

The Eq. (17) is a nonlinear equation where $\hat{\mathbf{K}}_S$ is a nonsymmetrical matrix dependent on the response $\hat{\mathbf{U}}$. Therefore, the Eq. (17) has to be solved iteratively.

2.2. Design sensitivity analysis

Consider now a general performance measure defined in the interval $[0, T]$ as

$$\Psi = \int G({}^t\mathbf{z}, {}^t\mathbf{b}, t) dt \quad (18)$$

where ${}^t\mathbf{b}$ is the vector of design and control variables and ${}^t\mathbf{z}$ is the state vector already defined in Eq. (15). The external forces ${}^t\mathbf{P}$ generally belong to the design vector.

The design sensitivity analysis problem is to derive the total design variation of the measure in Eq. (18) with respect to the design ${}^t\mathbf{b}$, for the system represented by the equation of motion, Eq. (17).

The total design variation for the performance measure of Eq. (18) is

$$\bar{\delta} \Psi = \bar{\bar{\delta}} \Psi + \tilde{\delta} \Psi \quad (19)$$

where $\bar{\bar{\delta}} \Psi$ and $\tilde{\delta} \Psi$ represent respectively the explicit and implicit (state dependent) design variations.

In order to formulate the adjoint system method, replace the arbitrary variation of state fields by adjoint fields into the virtual work equation as

$$W^a = (\hat{\mathbf{K}}_S \hat{\mathbf{U}} - \hat{\mathbf{P}}) \cdot \hat{\mathbf{U}}^a = 0 \quad (20)$$

and define an extended ‘action’ function

$$A = \Psi - W^a \quad (21)$$

The basic idea of introducing an adjoint system is to replace the implicit design variations of the state fields by explicit design variations and adjoint fields to be determined by imposing the implicit design variation of the ‘action’ function A to vanish,

$$\tilde{\delta} A = 0 \quad (22)$$

then the total design variation $\bar{\delta} \Psi$ can be written as

$$\bar{\delta} \Psi = \bar{\bar{\delta}} A \quad (23)$$

In order to solve the design sensitivity analysis problem, the sensitivities are firstly performed at the element level and then the sensitivity equations are assembled. The explicit design variation of the vector of element forces of Eq. (13) is

$$\bar{\bar{\delta}} \mathbf{R}^e = \bar{\bar{\delta}} \mathbf{P}^e - \bar{\bar{\delta}} \mathbf{F}^e, \quad \mathbf{F}^e = \mathbf{D}_S^e \mathbf{z}^e \quad (24)$$

and the implicit design variation of the internal forces gives

$$\tilde{\delta} \mathbf{F}^e = \tilde{\delta} \mathbf{D}_S^e \mathbf{z}^e + \mathbf{D}_S^e \tilde{\delta} \mathbf{z}^e = \mathbf{D}^e \tilde{\delta} \mathbf{z}^e \quad (25)$$

where

$$\mathbf{D}^e = \begin{bmatrix} {}^t\mathbf{K} & {}^t\mathbf{C} & \mathbf{M} & \mathbf{0}_{n \times n} & \mathbf{0}_{n \times n} & \mathbf{0}_{n \times n} \\ \hline {}^t\mathbf{D}_{11} & {}^t\mathbf{D}_{12} & \mathbf{0}_{n \times n} & {}^{t+\Delta t}\mathbf{D}_{11} & {}^{t+\Delta t}\mathbf{D}_{12} & \mathbf{0}_{n \times n} \\ {}^t\mathbf{D}_{21} & {}^t\mathbf{D}_{22} & \mathbf{0}_{n \times n} & {}^{t+\Delta t}\mathbf{D}_{21} & {}^{t+\Delta t}\mathbf{D}_{22} & \mathbf{0}_{n \times n} \end{bmatrix} \quad (26)$$

with

$$\begin{aligned} {}^t\mathbf{K} &= {}^t\mathbf{K}_S + {}^t\mathbf{K}_{S,U} {}^t\mathbf{U}, & {}^t\mathbf{C} &= {}^t\mathbf{C}_S + {}^t\mathbf{C}_{S,\dot{U}} {}^t\dot{\mathbf{U}} \\ {}^t\mathbf{D}_{jk} &= {}^t\mathbf{D}_{Sjk} + {}^t\mathbf{D}_{Sjk,U} {}^t\mathbf{U} + {}^t\mathbf{D}_{Sjk,\dot{U}} {}^t\dot{\mathbf{U}} \end{aligned} \quad (27)$$

Sensitivities of Eq. (24) and the element dynamic matrix of Eq. (26) are assembled and again imposed the time boundary conditions resulting respectively $\bar{\bar{\delta}} \hat{\mathbf{R}}$ and $\hat{\mathbf{K}}$.

Now, the application of the Eq. (22) to the Eq. (21) gives the adjoint system equilibrium

$$\hat{\mathbf{K}}^T \hat{\mathbf{U}}^a = (\Psi_{,\dot{U}})^T \quad (28)$$

Then, the total design variation of Eq. (23) is

$$\bar{\delta} \Psi = \bar{\bar{\delta}} \Psi + \hat{\mathbf{U}}^a \cdot \bar{\bar{\delta}} \hat{\mathbf{R}} \quad (29)$$

3. Example

A simple one degree of freedom system of unit mass is subject to a stochastic kinematic excitation specified by a maximum expected value of 50 mm and a maximum variation of 0.25 mm for the perturbation displacement. Considering the average absolute value of the acceleration along the time T as the optimality criterium, say $a_{avg} = T^{-1} \int |\ddot{u}| dt$, the system is optimized with respect to the spring and damping characteristics, respectively the stiffness K and the damping C . By using a Wiener process and trinomial trees of five steps, 12 point obstacles of different heights and probabilities of occurrence are determined. These obstacles are randomly distributed by Monte Carlo's method, according to the respective frequency of occurrence, along a time interval of $T = 50$ s, separated of each other by 0.05 s, originating 1000 random obstacles, defining a kinematic input. Additionally, five different Monte Carlo simulations were run for the same maximum expected value and maximum variation of the input. This way, we generated five different sets of obstacles.

The starting design variables were prescribed as $K = 100$ N/m and $C = 10$ N.s/m. With these values one achieves an objective of $1.3375 \leq a_{avg} \leq 1.4497$ m/s². The optimum values of the design variables are estimated as $91.93 \leq K^* \leq 94.76$ N/m, $92.68 \leq C^* \leq 101.3$ N.s/m by using a recursive quadratic algorithm. The optimum objective is estimated as $0.97553 \leq a_{avg}^* \leq 0.99161$ m/s² with a mean value of 0.97665 m/s², and a standard deviation of 0.014294 m/s², resulting a save at least of about 35%. In Fig. 1 one can see the starting and the optimum acceleration response for one the Monte Carlo simulations.

The optimization runs were then repeated with $T = 100$ s, instead of $T = 50$ s and the same time separation between obstacles, this way doubling its number. The mean and standard deviations of the average acceleration obtained for that second run of the optimizations were 0.95979 m/s^2 and 0.014676 m/s^2 , respectively. The small differences obtained lead us to conclude that the number of obstacles in the first run was already high enough to eliminate the influence of the number of obstacles in the optimization results.

4. Remarks

This paper presents an approach to optimize for designing of mechanical systems with random input parameters, where these parameters are characterized by specifying their probability density functions. As the integration of these functions is generally impracticable, a methodology is applied, where random parameters are described by generalized Wiener processes, such that it makes possible to substitute the continuous distributions by discrete distributions with identical expected values and covariance matrices. The system kinematic input is simulated by obstacles which dimensions are random. The obstacles distribution in time is simulated by using Monte Carlo's method. The optimization runs are performed accordingly to the Monte Carlo simulations, obtaining optimum values that improve effectively the objective function. For the optimization process, the design sensitivity analysis has been performed by using the adjoint system method. As the dynamic equations are integrated "at-once", this method does not have the same drawbacks as it has in the case of "step-by-step" integration, namely the need of memorizing the entire analysis response.

Fig. 2. Starting and optimum acceleration response for one the Monte Carlo simulations.

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